



Dee Why Bowling Club Flood Study Assessment Report

221 - 223 Fisher Road North

DEE WHY NSW 2099

Revision History

REVISION	DATE	BY	CHECKED	COMMENTS
A	12/07/19	AW	BM	Initial draft Issue
B	01/08/19	AW	BM	Issue for Approval

The recipient of the latest issue as noted above will be responsible for superseding/destroying all previous documents.

Contents

1.	Preliminary Information	4
1.1.	Site Characteristics	4
1.2.	Limitations	4
1.3.	Datasets	5
1.4.	Existing Flood Information	5
1.5.	Proposed Development	6
<hr/>		
2.	Hydrologic Modelling	7
2.1.	Catchment Extents	7
2.2.	WBNM Modelling	8
<hr/>		
3.	Hydraulic Modelling	11
3.1.	TUFLOW Modelling	11
3.2.	Proposed Model	12
3.3.	Model Results	12
3.4.	Hazard and Risk Assessment	13
3.5.	Assessment against DCP / LEP objectives	14
<hr/>		
4.	Conclusions & Recommendations	22
4.1.	Conclusions	22

1. Preliminary Information

The primary purpose of this report is to provide a flood assessment for the subject site. This report should be read in conjunction with the associated architectural design drawings and other development application documentation.

1.1. Site Characteristics

The subject site is located on the eastern side of Fisher Rd North, refer Figure 1. The site is currently occupied by 3 bowling greens and various ancillary buildings, including the clubhouse. The subject site comprises Lot 32 DP 868310. The total area occupied by the subject site is approximately 1.15ha. The site is relatively flat with slight fall to the southern corner. Several creeks / watercourses run around the site in a southerly direction towards Cromer Park. The catchment drained by these watercourses is relatively small and forms a part of the wider Dee Why Lagoon catchment.

Figure 1. "Site Location"



1.2. Limitations

All data, observations and opinions contained in this report pertain to hydraulic assessment of flood flows at or in the vicinity of the site. This report neither purports to be nor is an investigation into any other aspect of flooding within the site or surrounding catchment.

This report and the results contained within are only as accurate as the survey information provided. JN takes no responsibility or liability for incorrect survey information. The report is only valid for the development as proposed and detailed in this report and is not valid for any other design, layout or development.

1.3. Datasets

Hydrologic model inputs and all mapping presented in this report were undertaken using the GIS MapInfo & Global Mapper. These programs are geographical database systems that allow detailed cadastral, topological and flood data to be displayed and manipulated. Subarea data and other statistics were generated using inbuilt GIS functions and exported directly to the hydraulic model.

The datasets used to construct the models included:

- 2m contours & cadastre (LPI, Northern Beaches LGA);
- Aerial photography (May 2019, Nearmap);
- Detailed site & surrounds survey & 3D TIN (CMS Surveyors, Jan-Mar 2016);
- 1m ALS DEM (capture date April 2013) from LPI;
- Pipe data from Northern Beaches Council 'Stormwater assets' mapping layer;
- Site inspections, June 2019.

During our June 2019 site inspections, we undertook hand measurements of several of the pipe / culvert systems in the local vicinity, in order to bolster confidence in the data available on Council's mapping layer. In general, our measurements corresponded well with the data on Council's mapping layer.

The MGA56 coordinate system was used for datasets wherever possible.

1.4. Existing Flood Information

Council's adopted flood studies for the Dee Why Lagoon are:

- A. Dee Why & Curl Curl Lagoons Flood Study (LACE, 2002);
- B. Dee Why & Curl Curl Lagoons Floodplain Risk Management Study and Appendices (LACE, Nov 2005).

The mapping in the floodplain risk management study (FRMS) indicated that the LACE model boundary was limited to Cromer Park, and thus the subject site is excluded from Council's studies.

The FRMS states that "*In the lower reaches of the creeks, water levels are influenced by the storage in the lagoons, initial water levels and the prevailing entrance conditions prior to the commencement of surface runoff. Dee Why and Curl Curl Lagoons are examples of Intermittently Closed and Open Lakes and Lagoons (ICOLLS). On Dee Why Creek, the lagoon and the bridge over Pittwater Road influence flood levels for a distance of about 200 m upstream of Pittwater Road. Upstream of this location, flood levels are controlled by the hydraulic conveyance capacity of the channel and its overbank areas as well as the bed slope of the stream.*"

On this basis, we have assumed that water levels at the subject site (over 1km upstream of Pittwater Rd) are governed by the local hydraulic conveyance capacity of the watercourses, as opposed to tidal / lagoon dominated, as would be the case for areas located lower in the catchment.

1.5. Proposed Development

The development as proposed consists of demolition of the existing clubhouse, and construction of a new clubhouse with basement parking, and a seniors living unit block with basement parking, plus other ancillary (e.g. outdoor carparking, drainage, etc) works.

2. Hydrologic Modelling

2.1. Catchment Extents

The subject site is located within the Dee Why Lagoon catchment. The main flowpath in the local vicinity is southwards through the local creeks / watercourses that wrap around the site and drain towards the open channel (rectangular base, concrete lined) that drains along the southern side of the Cromer Park sports fields.

The local catchment extents and subarea boundaries are delineated in Figure 2; catchment statistics are detailed in table A.

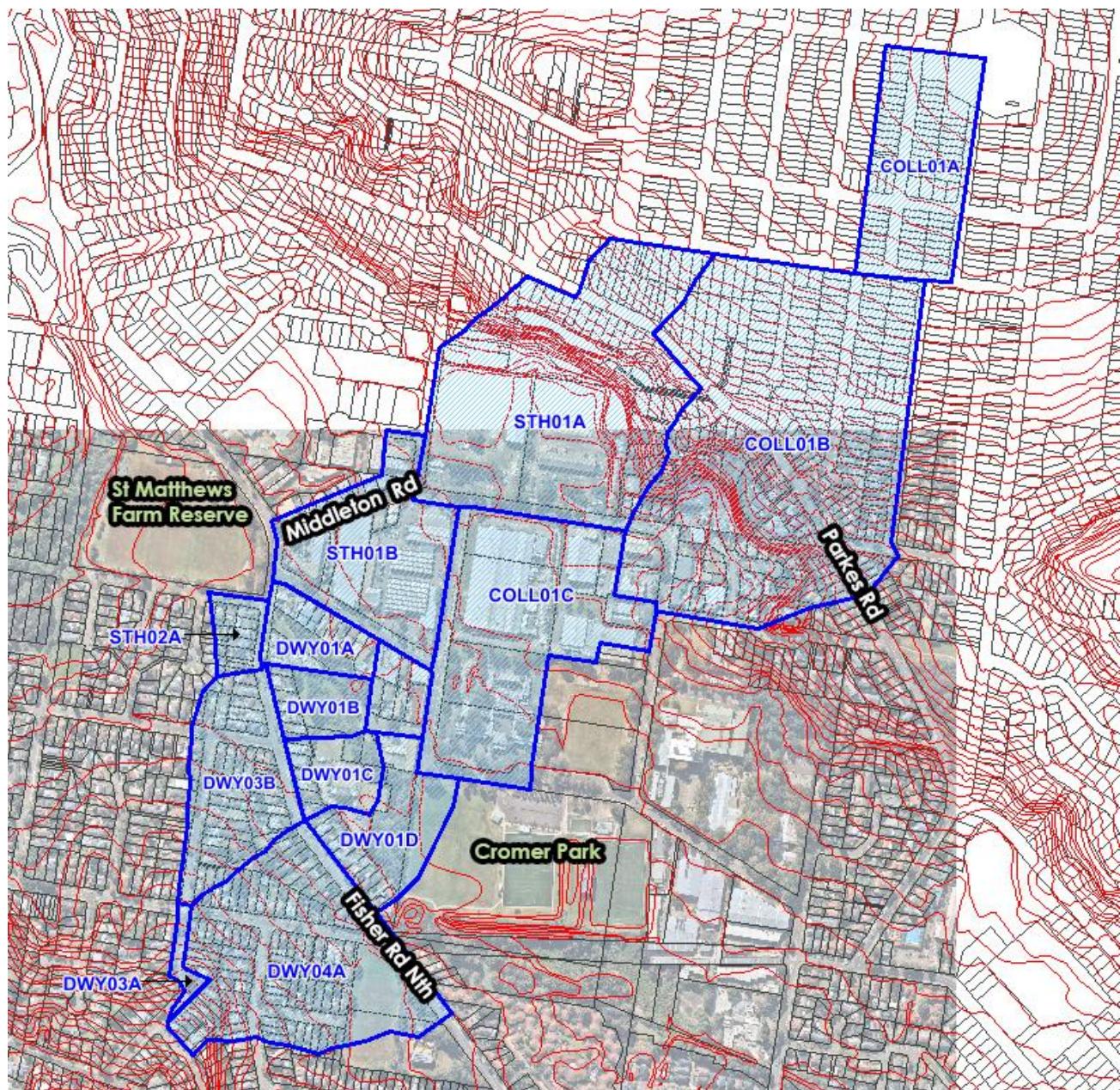
The catchment draining to the intersection of Fisher Rd North and Middleton Rd (Subareas STH 01A, 01B and 02A) has been reviewed during the site inspection and also using the ALS 1m DEM dataset. This review indicated that these 3 subareas are collected via a d1350mm pipe that runs northwards into St Matthews Farm reserve; flows in excess of the pipe capacity will pond in the sag point in Fisher Rd Nth, before spilling into St Matthews Farm reserve. It is unlikely that cross-catchment flow into the top of DWY01A will occur without large ponding depths in the sag point. This also appears to be the conclusion of Council's flood study and FRMS, which exclude this area from the Dee Hwy lagoon catchment (and instead place it within the South Creek catchment).

We have generally delineated subareas using the local pipe network as guidance, which also appears to the methodology in Council's flood study and FRMS; it is possible that roads such as Parkes Rd divert a portion of the upstream flow to other parts of the catchment. However, we have conservatively ignored these diversions, as a detailed analysis and/or catchment-scale 2D model is beyond the scope of this local flood study.

table A: Subarea Statistics

SUBAREA	AREA [HA]	IMPERVIOUS%
COLL01A	5.242	55
COLL01B	19.802	60
COLL01C	9.319	55
STH01A	13.396	60
STH01B	5.742	95
DWY01A	1.707	55
STH02A	0.889	55
DWY02A	1.092	95
DWY01B	1.436	15
DWY03A	0.389	90
DWY03B	4.448	55
DWY01D	3.027	25
DWY01C	1.536	15
DWY04A	9.169	50

Figure 2. "Catchment Extents"



2.2. WBNM Modelling

Hydrological Modeling was undertaken using iWBNM 2013, an updated interface to the WBNM hydrological engine. WBNM ('Watershed Bounded Network Model' Boyd et al, 2007) is an advanced storage-routing model developed in conjunction with the University of Wollongong that allows simulation of complex catchment behaviour.

A summary of the adopted modelling parameters is provided below.

- Lag Parameter: 1.60 (recommended default)
- Stream Routing: N/A (typically performed in TUFLOW)
- Initial Loss: 0.00 [mm] (to account for antecedent conditions)
- Continuing Loss Rate: 0.72 [mm/hr] (as per ARR2016, see note below)
- Raingauges: Dee Why (1 total, ARR2016)

The ARR datahub specifies a loss rate of 1.8 mm/hr for the local area but notes that this may not be applicable in urban areas; without any further information on loss rates we have applied the 0.4 factor (= 0.72) as recommended on the ARR datahub website. Model results are presented in their entirety in the included attachment; table B reproduces the most relevant values.

Australian Rainfall Runoff (ARR) 2016

ARR2016 was released in late-2016, although as of 2019 some final chapters are still missing. We have adopted the ARR2016 procedure for hydrograph estimation for the 100yr ARI (1% AEP) event, noting that ARR2016 specifies that an ensemble of 10 patterns be run for every duration, with the design peak flow taken as the average of the 10 patterns. With respect to input hydrographs, it is recommended that the pattern with a peak flow just above the mean be adopted. ARR2016 data is available on the associated Datahub website; we have adopted loss rates from the datahub as noted above. IFD2016 depth data was also used (taken from the BOM website) for a gauge in Dee Why.

WBNM Results & Critical Duration Analysis

WBNM results are presented in their entirety in the appendices and in summary below. The ARR2016 duration with the highest flowrate for the 1%AEP event was 20 minutes. As a check, the following ARR2016 1%AEP durations were run in a preliminary hydraulic model: 20 / 30 / 60 minutes. These checks indicated the 30-minute durations produced the highest water levels at the subject site. We have therefore adopted the 30-minute duration storm for hydraulic modelling for the 1%AEP event. A full spectrum of durations (run hydraulically in 2D) was not feasible for this small-scale flood study; while it is possible that another duration produces the highest level at the subject site, it is highly unlikely that the resulting flood level would be significantly higher than the levels calculated.

The following pattern numbers were selected as being representative of the ‘average’ flowrate value; that is, their peak flowrates were at or slightly above the calculated average value for most subareas.

- 1%AEP 20min 4428
- 1%AEP 30min 4504
- 1%AEP 60min 4463

The hydrographs from the above storm patterns were converted to TUFLOW TS1 files and used as direct model input for the TUFLOW hydraulic model.

The PMF 20-minute storm produced the highest flowrates at the subject site in WBNM; we have checked the 20 / 60 / 90 minute storms hydraulically, and the 60-minute produced the highest water

level at the subject site, noting that the PMF flood levels are storage-driven, with ponding behind the Cromer Sports field embankments due to the inability of the large flows to escape the fields in an efficient manner. We have therefore adopted the 60-minute PMF storm for analysis purposes.

table B: WBNM Results – Select subareas, Q_{peak} [m^3/s]

SUBAREA	1%AEP 20MIN	1%AEP 30MIN	1%AEP 60MIN
DWY04A	3.817	3.435	2.773
DWY03A	0.228	0.196	0.145
DWY03B	2.200	1.980	1.561
DWY01A	0.854	0.766	0.583
DWY01B	1.307	1.190	0.961
DWY02A	0.611	0.534	0.397
DWY01C	4.193	3.876	3.155

3. Hydraulic Modelling

3.1. TUFLOW Modelling

TUFLOW is a 1D / 2D grid model that performs hydro-dynamic analysis in river and floodplain systems. TUFLOW allows the user to model a range of structures common in urban environments. The 64-bit double precision version of TUFLOW (2013 AC) was used as the hydraulic model for this report.

TUFLOW model parameters were as follows:

- Grid cell size: 0.75m
- 2D Timestep: 0.2s
- 1D Timestep: 1.0s

Additional parameters were generally left at the recommended default values. A downstream outflow boundary was located in the channel adjacent to Cromer Park and TUFLOW was allowed to calculate the water level at this location.

The following cases were modelled:

1. Existing case;
2. Proposed case: incorporating various changes as discussed in Section 3.2.

Material zones and roughness values are summarised in table C. The model extents and material zones are depicted in the figures at the end of this chapter.

table C: Manning's N-values

MATERIAL	N-VALUE
Channel - concrete	0.015
Channel – dense vegetation	0.080
Dense trees / vegetation	0.085
Industrial yards [a]	0.020
Road reserve	0.025
Short grass / sports fields	0.035
Urban yard [a]	0.060
Buildings	Raised

[a] yards exclude buildings, which are separately raised in model

Pit & Pipe network & Blockage parameters

We have modelled 2 of the larger sag pits in Fisher Rd North; in all scenarios these pits were modelled with a 50% inlet blockage factor. We have considered the culverts / pipes within the model to be either all-clear or 50% blocked depending on the scenario analysed. In general, the 50% blocked case only resulted in marginally higher (+100-200mm) water levels in the vicinity of the proposed development at the peak of the 1%AEP event.

3.2. Proposed Model

The proposed model incorporated the following changes:

- A. The new building outlines (clubhouse, seniors living unit block) were input as solid obstructions;
- B. We have assumed the area behind the retaining wall near Unit#20 of the Seniors Living unit will be filled to RL +10.80.

We highlight that the proposed development outline is almost entirely located outside of the 1%AEP flood extents, and as such, there are no significant conveyance impacts or loss of flood storage during the 1%AEP event or smaller.

3.3. Model Results

Mapping is included in Appendix B. We highlight the following:

- A. The subject site is almost entirely flood free in the 1%AEP flood event under both all-clear and 50% blocked scenarios;
- B. 1%AEP flood levels vary along the watercourses, from around +10.37 mAHD (north-west site corner) to +10.35 mAHD (entrance to twin d900mm pipes under bowling green) to +10.07 mAHD (southern site corner);
- C. 1%AEP flooding is typified by conveyance-controlled flows in the local waterways; the twin d900mm RCP's running beneath the bowling club have sufficient capacity to convey the 1%AEP flows even in the 50% blocked condition;
- D. Under 50% blocked scenario, the 1%AEP levels upstream of the site increase by around +150mm, and by around +70mm downstream of the site;
- E. 1%AEP flow velocities in the channels and overbanks areas surrounding the site are typically very small, less than 0.5 m/s. The exception to this is the concrete channels in or adjacent to Cromer Park, where velocities are around 2-3 m/s;
- F. PMF flooding is typified by ponding behavior in Cromer Park, with insufficient ability for the very large flows to escape the oval embankments;
- G. The maximum PMF ponding level at the site is +11.25 mAHD.

Conveyance Impacts

The development is located almost entirely outside of the 1%AEP flood extents, and as such, there are no offsite impacts in the 1%AEP flood event as demonstrated by our modelling.

PMF flooding is typified by ponding at very low velocities, and while the proposed development is a solid obstruction to PMF flows, the model indicated very small (less than 20mm) impacts on the surrounding adjacent lots.

Flood Storage Impacts

The proposed development footprint only extends slightly into the 1%AEP flood extents in an area of shallow ponding (50-100mm flow depth). We estimate a flood storage loss of 5-10m³ in the 1%AEP event which is insignificant and will have no measurable impact on hydrograph attenuation due to loss of storage routing. This can be readily offset through minor excavation works.

Floor Level Recommendations

With respect to floor levels, we recommend the following:

- A. The Clubhouse currently incorporates a ground floor level at RL +12.95 mAHD, well above the flood planning level (FPL, approximately +10.9 mAHD for the subject site, being 500mm above the highest 1%AEP level) and indeed PMF level;
- B. There is no requirement for the Clubhouse basement level to be flood protected to any level, based on the Warringah DCP 2011 flood matrix assessed in Section 3.5 following;
- C. The Seniors Living Development is a 'vulnerable use' and must be set at the higher of the FPL and the PMF, this equates to RL +11.25 mAHD (PMF) and all habitable floors meet or exceed this requirement;
- D. The Seniors Living basement is required to be flood protected to the PMF level (+11.25 mAHD); this can only be achieved through the use of a flood barrier such as Flow Defense that automatically activates when flood levels reach a set trigger level. Further comment is provided in section 3.5.

3.4. Hazard and Risk Assessment

NSW FDM Hazard Assessment

The NSW Floodplain Development Manual (2005) provides guidelines for determining the hydraulic flood hazard at a site. A provisional hazard can be assigned to an area using Figure L2 and the combined impact of flood velocity and flood depth.

In general, an area will be (provisionally) assigned High Hazard if any of the following criteria are satisfied:

- The flood depth (D) is greater than 1.0 m;
- The flood velocity (V) is greater than 2.0 m/s;
- The combination of V and D lie in the dark blue region (mathematically this is approximately where $V + 3.33D$ is greater than 3.33).

Under both existing and proposed conditions, the areas of (provisional) High Hazard in the 1%AEP event are limited to the creek channels themselves, with the remaining overbank areas almost entirely Low Hazard. We note there is no change to these categories under proposed conditions.

ARR2016 Hazard Assessment

ARR2016 provides updated Hazard curves as described in Table 6.7.3 and 6.7.4 of ARR2016 Chapter 6. We have provided mapping of the 6 hazard categories, with the definitions as follows:

- H1: Generally safe for vehicles, people and buildings.
- H2: Unsafe for small vehicles.
- H3: Unsafe for vehicles, children and the elderly.
- H4: Unsafe for vehicles and people.
- H5: Unsafe for vehicles and people. All buildings vulnerable to structural damage. Some less robust buildings subject to failure.
- H6: Unsafe for vehicles and people. All building types considered vulnerable to failure.

Under both existing and proposed conditions creek channels are predominantly H4-H5 category at the peak of the 1%AEP event. The flooded areas upstream of the site outside of the channels themselves are predominantly H1-H2, and the flooded area downstream of the site is mostly H3.

Cromer Park is a combination of H1-H3. We note there is no change to these categories under proposed conditions.

Risk Assessment

As far as we are aware, the Warringah DCP 2011 still applies to the subject site. The Warringah definitions of flood risk are related to the NSW FDM hazard classifications described earlier and are as follows:

High Risk: within 1%AEP flood extents and subject to High Hazard, or within the floodway or subject to significant evacuation difficulties;

Medium Risk: within 1%AEP flood extents and not High Hazard;

Low Risk: not within the 1%AEP flood extents but subject to PMF flooding.

Based on the above, the subject site is almost entirely **Low Flood Risk** with very small / isolated areas of Medium Flood Risk.

3.5. Assessment against DCP / LEP objectives

The proposed development incorporates a Seniors Living development, which falls into Warringah DCP 2011 “vulnerable uses” category. The flood risk category that most applies to the development footprint is Low Risk, and we have assessed the proposal against the relevant column in the Warringah DCP flood matrix, refer also Table D. We have assessed the clubhouse portion of the proposed development separately as ‘Business / Commercial’.

Table D: Assessment against DCP 2011: Seniors Living Portion of Development

DCP OBJECTIVE	COMMENT
Flood effects: A2 A3 A4 Certification shall be provided in accordance with Northern Beaches Council's Standard Hydraulic Certification Form (Forms A and A1 of Northern Beaches Council's Guidelines for preparing a Flood Management Report) to the effect that the works have been designed and can be constructed to adequately address flood risk management issues. The applicant shall include in their submission, calculations to illustrate that any fill or other structures that reduce the total flood storage are replaced by Compensatory Works. Development (including earthworks and subdivision) shall not be approved unless it can be demonstrated in a Flood Management Report that it has been designed and can be constructed so that in a Probable Maximum Flood event: (a) There are no adverse impacts on flood levels and velocities caused by alterations to the flood conveyance; (b) There are no adverse impacts on surrounding properties; and (c) It is sited to minimise exposure to flood hazard.	We believe the flood risk management issues at the subject site can be satisfactorily addressed; certification can be readily provided at the CC stage. There is a nominal loss of flood storage in the 1%AEP flood event of 5-10m ³ and this can be readily offset via compensatory excavation. PMF flooding in the area surrounding the site is characterised by ponding behind the Cromer Park embankments. As such, the proposed hydraulic model indicates that there are limited impacts on offsite properties with respect to water level or velocity increases during a PMF event. Water level increases upstream of the site are no more than +20mm, which we believe is acceptable.
Drainage infrastructure: B1 B2 Flood mitigation works or stormwater devices that modify a major drainage system, stormwater system, natural water course, floodway or flood behaviour within or outside the development site may be	There are currently no instream works or other flood mitigation works proposed.

<p>permitted subject to demonstration through a Flood Management Report that they comply with the Flood Prone Land Design Standard found on Council's webpage.</p> <p>A Section 88B notation under the Conveyancing Act 1919 may be required to be placed on the title describing the location and type of flood mitigation works with a requirement for their retention and maintenance.</p>	
<p>Building Components: C1 C2 C3</p> <p>All buildings shall be designed and constructed as flood compatible buildings in accordance with Reducing Vulnerability of Buildings to Flood Damage: Guidance on Building in Flood Prone Areas, Hawkesbury-Nepean Floodplain Management Steering Committee (2006).</p> <p>All structures must be designed and constructed to ensure structural integrity up to the Flood Planning Level, taking into account the forces of floodwater, wave action, flowing water with debris, buoyancy and immersion. Structural certification shall be provided confirming the above. Where shelter-in-place refuge is to be provided the structural integrity is to be to the Probable Maximum Flood level.</p> <p>All new electrical equipment, power points, wiring, fuel lines, sewerage systems or any other service pipes and connections must be waterproofed and/or located above the Flood Planning Level. All existing electrical equipment and power points located below the Flood Planning Level must have residual current devices installed that turn off all electricity supply to the property when flood waters are detected.</p>	<p>Structural certification can be readily provided at the CC design stage. The structural integrity should be certified to the PMF level of +11.25 mAHD.</p>
<p>Storage: D1 D2</p> <p>Hazardous or potentially polluting materials shall not be stored below the Flood Planning Level unless adequately protected from floodwaters in accordance with industry standards.</p> <p>Goods, materials or other products which may be highly susceptible to water damage are to be located/stored above the Flood Planning Level.</p>	<p>As far as we are aware, there are no proposed hazardous or polluting goods to be stored as part of the Seniors Living development.</p> <p>All habitable floors will be located well above the Flood Planning Level of +10.9 mAHD.</p>
<p>Flood emergency response: E1 E2 E3</p> <p>Development shall comply with Council's Flood Emergency Response Planning for Development in Pittwater Policy and the outcomes of any Flood Risk Emergency Assessment Report where it applies to the land.</p> <p>New development must provide an appropriately sized area to safely shelter in place above the Probable Maximum Flood level and appropriate access to this area should be available from all areas within the development.</p>	<p>A flood emergency response plan may be readily prepared as part of the CC documentation.</p> <p>With respect to evacuation, we believe the safest option is for all residents to shelter-in-place, noting that all habitable floor levels will be at or above the PMF flood level.</p> <p>Appropriate signage should be prepared advising residents to remain within their dwellings during a flood event, and await for the SES or other emergency personnel to give direction. This signage should be placed in the ground floor lobbies and at the ground</p>

<p>Adequate Warning Systems, Signage and Exits shall be installed to allow safe and orderly evacuation without reliance upon the SES or other authorised emergency services personnel.</p>	<p>floor exits. We do not believe that offsite flood evacuation should be undertaken in any circumstance, unless directed by the SES.</p>
<p>Floor Levels: F2 F3 F7</p> <p>All development structures must be designed and constructed so as not to impede the floodway or flood conveyance on the site, as well as ensuring no loss of flood storage in a 1% AEP Event. Where the dwelling is located over a flow path it must be elevated on suspended pier/pile footings such that the level of the underside of all floors including balconies and decks within the flood affected area are at or above, or raised to the Flood Planning Level to allow clear passage of the floodwaters under the building. The development must comply with the Flood Prone Land Design Standard.</p>	<p>The development as proposed is almost entirely located in an area that is flood free in the 1%AEP event, and as such the development does not impede flood flows or affect the conveyance capacity of the local waterways during this event. There is currently no proposal to elevate any portion of the Seniors Living unit to allow floodwaters through or beneath the building.</p>
<p>Where the lowest floor has been elevated to allow the passage of flood waters, a restriction shall be imposed on the title of the land, pursuant to S88B of the Conveyancing Act confirming that the undercroft area is not to be enclosed.</p>	<p>There is a nominal loss of flood storage in the 1%AEP flood event of 5-10m³ and this can be readily offset via compensatory excavation.</p>
<p>All floor levels within the development shall be at or above the Probable Maximum Flood level or Flood Planning Level whichever is higher.</p>	<p>All habitable floor levels of the Seniors Living unit will be at or above the PMF level of +11.25 mAHD, noting this is higher than the FPL (+10.9 mAHD).</p>
<p>Car Parking: G2 G6 G7 G9 G10</p> <p>The lowest floor level of open carparks and carports (unroofed or with open sides) shall be constructed no lower than the natural ground levels</p>	<p>There are no proposed carparks or enclosed garages as part of the Seniors Living development.</p>
<p>Carports must comply with the Flood Prone Land Design Standard</p> <p>Where a driveway is required to be raised it must be demonstrated that there is no loss to flood stage in the 1% AEP flood event and no impact on flood conveyance through the site.</p>	<p>Several visitor car spaces will be located as depicted on the architectural plans, and these will be set at natural (existing) ground levels.</p>
<p>All enclosed car parks must be protected from inundation up to the Probable Maximum Flood level or Flood Planning Level whichever is higher. For example, basement carparks must be provided with a crest at the entrance, the crest of which is at the relevant Probable Maximum Flood level or Flood Planning Level whichever is higher. All access, ventilation and any other potential water entry points to any enclosed car parking shall be above the relevant Probable Maximum Flood level or Flood Planning Level whichever is higher.</p>	<p>The enclosed basement garage for the Seniors Living development has an access ramp that matches in to existing ground levels around RL +10.5 mAHD. Given a PMF level of +11.25, it is clear that a crest will be not be feasible solution for basement protection. However, we note that flood gates have been adopted in several Sydney LGAs as a solution to the ingress of floodwaters. We note that the Flow Defence (or similar flood barrier) are automatically activated using water pressure from the incoming floodwaters and do not require electronics. Maintenance requirements are minimal. We believe this is a robust and suitable solution to offer PMF flood protection to the basement level carpark.</p>
<p>Enclosed Garages must be located at or above the Probable Maximum Flood Level or Flood Planning Level whichever is higher.</p>	
<p>Fencing: H1</p> <p>Fencing, including pool fencing, shall be designed so as not to impede the flow of flood waters and not to</p>	<p>We recommend that all proposed fencing be flood compatible.</p>

<p>increase flood affectation on surrounding land. Appropriate fencing must comply with the Flood Prone Land Design Standard in addition to other regulatory requirements of pool fencing.</p>	
<p>Pools: I1</p> <p>Pools located within the 1% AEP flood extent are to be in-ground, with coping flush with natural ground level. Where it is not possible to have pool coping flush with natural ground level, it must be demonstrated that the development will result in no net loss of flood storage and no impact on flood conveyance on or from the site.</p>	<p>Not applicable: there are no pools proposed.</p>
<p>All electrical equipment associated with the pool (including pool pumps) is to be waterproofed and/or located at or above the Flood Planning Level.</p>	
<p>All chemicals associated with the pool are to be stored at or above the flood planning level.</p>	

table E: Assessment against DCP 2011: Clubhouse Portion of Development

DCP OBJECTIVE	COMMENT
<p>Floor Levels F1 F3 F8</p> <p>New floor levels within the development shall be at or above, the Flood Planning Level. A reduced Flood Planning Level may be considered only where it is permitted in this Development Control Plan. The structure must be flood proofed (wet or dry) to the Flood Planning Level. This control cannot be applied to critical or vulnerable uses.</p> <p>Where the lowest floor has been elevated to allow the passage of flood waters, a restriction shall be imposed on the title of the land, pursuant to S88B of the Conveyancing Act confirming that the undercroft area is not to be enclosed.</p> <p>The minimum floor level of any first-floor additions shall be at or above the Probable Maximum Flood Level.</p>	<p>The current proposed ground floor level at RL +12.95 mAHD is well above the FPL and PMF flood level.</p> <p>There are currently no areas of the clubhouse proposed to be elevated above floodwaters.</p> <p>There are no first-floor additions proposed as part of the clubhouse works.</p>

Figure 3. "Model Extents"

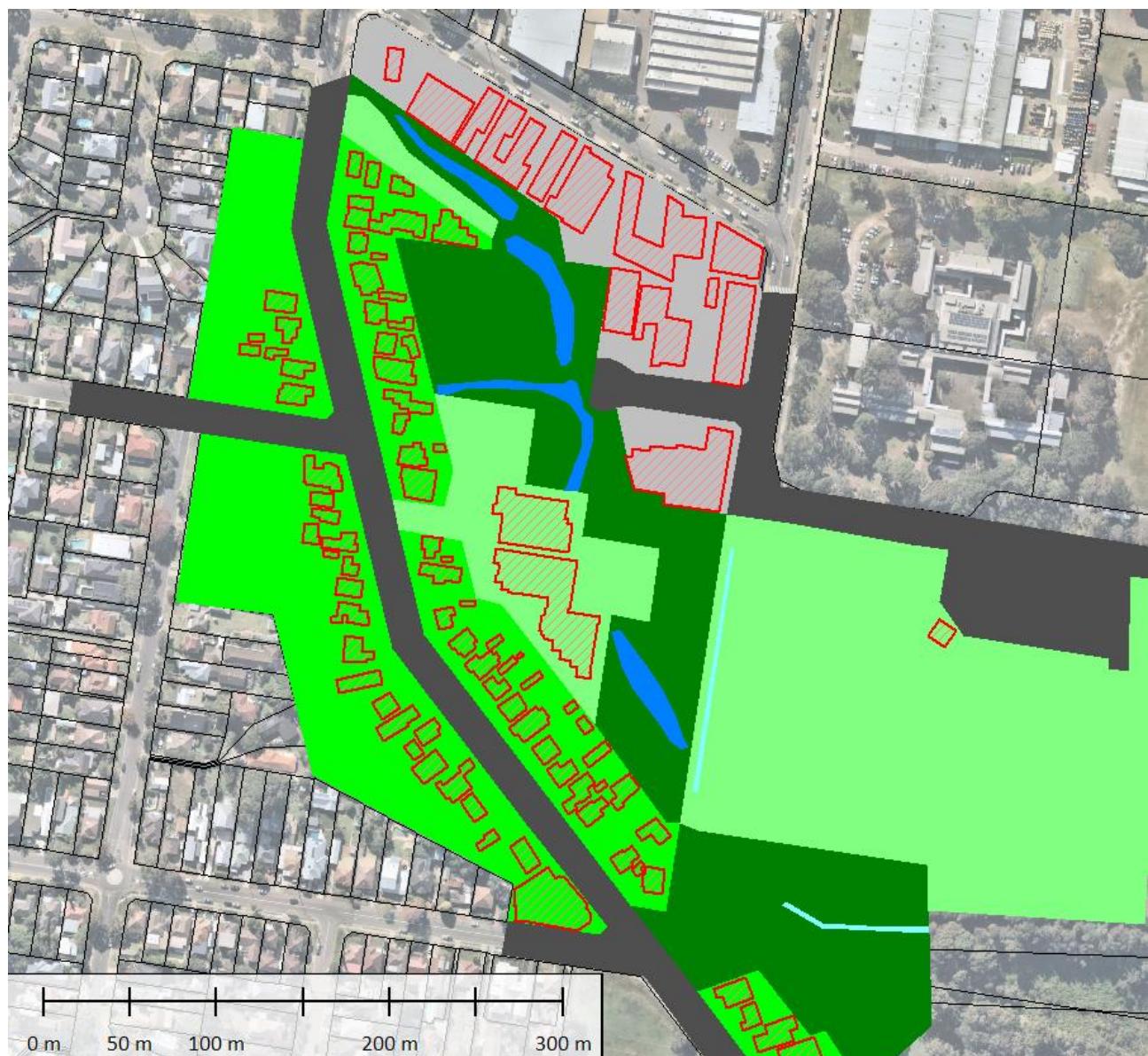


Figure 4. Existing Material Zones



[light grey = industrial yards, dark grey = roads, light green = short grass, bright green = urban yards, dark green = dense vegetation, light blue = channel – conc, dark blue =channel – veg, red hatch = buildings]

Figure 5. Proposed Material Zones



[light grey = industrial yards, dark grey = roads, light green = short grass, bright green = urban yards, dark green = dense vegetation, light blue = channel – conc, dark blue =channel – veg, red hatch = buildings]

4. Conclusions & Recommendations

4.1. Conclusions

We conclude that:

- A. The subject site is located in the Dee Why Lagoon floodplain. The site itself is almost entirely flood free in the 1%AEP flood event. Following from this, the development as proposed is situated almost entirely outside of the 1%AEP flood extents and there are no modelled conveyance impacts and only a very minimal loss of flood storage for this event;
- B. The flooded area surrounding the site is High Hydraulic hazard in the channel areas, and otherwise Low Hazard. The most appropriate category for the site as a whole is Low Flood Risk;
- C. It is advised that all the floor levels that are at RL +10.8 be raised to the PMF flood level of +11.25
- D. We believe the flood risk for the proposed development can be satisfactorily managed as noted in Section 3.5 of this report.

For and on behalf of JN,



Andrew Wiersma
BE (Hons) MEng, MIE(Aust), CPEng (NPER)
Senior Design Engineer



Appendix 1

Supplementary Information

IFD2016_Cromer.txt

CROMER PARK: 33.7375, 151.2875

Annual Exceedance Probability (AEP)

Duration	63.2%	50%#	20%*	10%	5%	2%	1%
1 min	2.45	2.74	3.67	4.32	4.97	5.86	6.56
2 min	4.09	4.51	5.87	6.81	7.71	8.96	9.94
3 min	5.65	6.26	8.18	9.52	10.8	12.6	14.0
4 min	7.07	7.85	10.4	12.1	13.8	16.2	18.0
5 min	8.36	9.30	12.3	14.5	16.6	19.5	21.8
10 min	13.3	14.8	20.0	23.6	27.2	32.2	36.1
15 min	16.6	18.6	25.1	29.7	34.3	40.5	45.4
20 min	19.1	21.4	28.8	34.0	39.3	46.4	52.0
25 min	21.1	23.7	31.7	37.4	43.1	50.9	57.0
30 min	22.8	25.5	34.1	40.1	46.2	54.4	60.9
45 min	26.6	29.6	39.3	46.1	52.9	62.1	69.4
1 hour	29.4	32.6	43.0	50.3	57.6	67.6	75.4
1.5 hour	33.5	37.1	48.5	56.6	64.6	75.7	84.5
2 hour	36.8	40.6	52.9	61.5	70.3	82.4	92.0
3 hour	41.9	46.1	59.9	69.8	79.8	93.7	105
4.5 hour	48.1	52.9	68.8	80.3	92.1	109	122
6 hour	53.2	58.6	76.5	89.6	103	122	138
9 hour	61.9	68.4	90.1	106	123	146	166
12 hour	69.3	76.8	102	121	141	168	191
18 hour	81.6	91.0	123	146	171	206	234
24 hour	91.7	103	140	168	197	237	270
30 hour	100	113	155	187	220	264	300
36 hour	108	122	168	203	239	287	326
48 hour	120	136	190	229	270	324	366
72 hour	137	156	219	265	311	372	419
96 hour	148	169	237	286	335	399	447
120 hour	156	178	248	297	347	412	462
144 hour	161	183	253	302	353	418	468
168 hour	164	185	255	303	354	419	469

ARR2016.txt

Results - ARR Data Hub
[STARTTXT]

Input Data Information

[INPUTDATA]

Latitude,-33.739000

Longitude,151.284000

[END_INPUTDATA]

River Region

[RIVREG]

Division,South East Coast (NSW)

River Number,13

River Name,Sydney Coast-Georges River

[RIVREG_META]

Time Accessed,29 May 2019 01:22PM

Version,2016_v1

[END_RIVREG]

ARF Parameters

[LONGARF]

Zone,SE Coast

a,0.06

b,0.361

c,0.0

d,0.317

e,8.11e-05

f,0.651

g,0.0

h,0.0

i,0.0

[LONGARF_META]

Time Accessed,29 May 2019 01:22PM

Version,2016_v1

[END_LONGARF]

Storm Losses

[LOSSES]

ID,29835.0

Storm Initial Losses (mm),41.0

Storm Continuing Losses (mm/h),1.8

[LOSSES_META]

Time Accessed,29 May 2019 01:22PM

Version,2016_v1

[END_LOSSES]

Temporal Patterns

[TP]

code,ECsouth

Label,East Coast South

ARR2016.txt

[TP_META]

Time Accessed,29 May 2019 01:22PM

Version,2016_v2

[END_TP]

Areal Temporal Patterns

[ATP]

code,ECsouth

arealabel,East Coast South

[ATP_META]

Time Accessed,29 May 2019 01:22PM

Version,2016_v2

[END_ATP]

Median Preburst Depths and Ratios

[PREBURST]

min (h)\AEP(%),50,20,10,5,2,1

60 (1.0),7.0 (0.214),7.7 (0.179),5.6 (0.111),3.5 (0.061),2.1 (0.031),1.0 (0.013)

90 (1.5),13.2 (0.355),9.5 (0.195),6.5 (0.114),3.6 (0.056),2.1 (0.028),1.0 (0.012)

120 (2.0),12.8 (0.316),7.7 (0.145),6.6 (0.108),5.6 (0.080),4.2 (0.051),3.2 (0.034)

180 (3.0),5.3 (0.116),6.5 (0.109),6.3 (0.091),6.2 (0.077),6.9 (0.074),7.4 (0.071)

360 (6.0),7.3 (0.124),11.0 (0.143),13.4 (0.149),15.7 (0.152),18.9 (0.155),13.0 (0.095)

720 (12.0),6.2 (0.081),11.0 (0.107),14.1 (0.116),17.1 (0.121),26.1 (0.155),30.3 (0.159)

1080 (18.0),1.8 (0.020),9.3 (0.076),14.3 (0.098),19.1 (0.111),22.2 (0.108),28.3 (0.121)

1440 (24.0),1.8 (0.017),5.6 (0.040),8.1 (0.048),10.5 (0.053),21.1 (0.089),26.9 (0.100)

2160 (36.0),0.0 (0.000),1.8 (0.010),2.9 (0.014),4.0 (0.017),9.4 (0.033),11.5 (0.035)

2880 (48.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),1.0 (0.003),1.7 (0.005)

4320 (72.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),1.2 (0.003),2.2 (0.005)

[PREBURST_META]

Time Accessed,29 May 2019 01:22PM

Version,2018_v1

Note,Preburst interpolation methods for catchment wide preburst has been slightly altered. Point values remain unchanged.

[END_PREBURST]

10% Preburst Depths

[PREBURST10]

min (h)\AEP(%),50,20,10,5,2,1

60 (1.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)

90 (1.5),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)

120 (2.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)

180 (3.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)

360 (6.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)

720 (12.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)

1080 (18.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)

1440 (24.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)

ARR2016.txt

2160 (36.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)
2880 (48.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)
4320 (72.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)

[PREBURST10_META]

Time Accessed,29 May 2019 01:22PM

Version,2018_v1

Note,Preburst interpolation methods for catchment wide preburst has been slightly altered. Point values remain unchanged.

[END_PREBURST10]

25% Preburst Depths

[PREBURST25]

min (h)\AEP(%),50,20,10,5,2,1
60 (1.0),0.0 (0.000),0.2 (0.005),0.1 (0.002),0.0 (0.000),0.0 (0.000),0.0 (0.000)
90 (1.5),0.0 (0.001),0.5 (0.010),0.2 (0.004),0.0 (0.000),0.0 (0.000),0.0 (0.000)
120 (2.0),0.1 (0.003),0.1 (0.001),0.0 (0.001),0.0 (0.000),0.0 (0.000),0.0 (0.000)
180 (3.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)
360 (6.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.4 (0.003),0.0 (0.000)
720 (12.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)
1080 (18.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),1.2 (0.006),2.2 (0.009)
1440 (24.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.4 (0.002),0.7 (0.003)
2160 (36.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)
2880 (48.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)
4320 (72.0),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000),0.0 (0.000)

[PREBURST25_META]

Time Accessed,29 May 2019 01:22PM

Version,2018_v1

Note,Preburst interpolation methods for catchment wide preburst has been slightly altered. Point values remain unchanged.

[END_PREBURST25]

75% Preburst Depths

[PREBURST75]

min (h)\AEP(%),50,20,10,5,2,1
60 (1.0),40.6 (1.247),37.9 (0.882),36.4 (0.723),34.9 (0.607),28.4 (0.420),23.5 (0.311)
90 (1.5),36.7 (0.989),39.7 (0.819),35.1 (0.620),30.6 (0.474),27.8 (0.367),25.7 (0.304)
120 (2.0),53.4 (1.316),37.7 (0.713),34.8 (0.565),32.0 (0.455),35.9 (0.435),38.7 (0.421)
180 (3.0),33.4 (0.724),45.5 (0.760),43.8 (0.628),42.2 (0.529),57.0 (0.608),68.1 (0.649)
360 (6.0),45.2 (0.771),54.8 (0.716),61.1 (0.682),67.2 (0.651),86.5 (0.708),90.0 (0.654)
720 (12.0),30.1 (0.392),43.5 (0.426),52.3 (0.433),60.8 (0.432),67.0 (0.398),76.8 (0.403)
1080 (18.0),22.9 (0.251),34.7 (0.282),42.5 (0.290),50.0 (0.292),71.4 (0.347),82.2 (0.352)
1440 (24.0),24.9 (0.243),31.9 (0.227),36.4 (0.217),40.8 (0.207),67.1 (0.283),74.8 (0.278)

ARR2016.txt

2160 (36.0),5.0 (0.041),15.4 (0.091),22.3 (0.110),28.9 (0.121),51.5 (0.179),60.1 (0.184)

2880 (48.0),12.3 (0.090),12.5 (0.066),12.7 (0.055),12.8 (0.048),22.7 (0.070),30.2 (0.082)

4320 (72.0),0.0 (0.000),0.3 (0.001),0.5 (0.002),0.7 (0.002),20.9 (0.056),33.8 (0.081)

[PREBURST75_META]

Time Accessed,29 May 2019 01:22PM

Version,2018_v1

Note,Preburst interpolation methods for catchment wide preburst has been slightly altered. Point values remain unchanged.

[END_PREBURST75]

90% Preburst Depths

[PREBURST90]

min (h)\AEP(%),50,20,10,5,2,1

60 (1.0),95.5 (2.929),94.8 (2.203),96.9 (1.926),98.9 (1.718),103.6 (1.533),107.1 (1.420)

90 (1.5),72.3 (1.949),105.2 (2.168),103.3 (1.825),101.4 (1.568),113.6 (1.500),122.8 (1.453)

120 (2.0),88.3 (2.178),94.6 (1.790),98.1 (1.595),101.5 (1.444),108.2 (1.314),113.2 (1.231)

180 (3.0),86.3 (1.871),102.2 (1.706),108.8 (1.559),115.1 (1.442),126.6 (1.351),135.2 (1.289)

360 (6.0),78.0 (1.331),89.9 (1.175),97.7 (1.091),105.2 (1.020),158.1 (1.295),175.3 (1.275)

720 (12.0),62.3 (0.811),86.2 (0.844),102.0 (0.844),117.2 (0.833),133.6 (0.794),148.3 (0.778)

1080 (18.0),47.4 (0.521),64.9 (0.529),76.5 (0.522),87.6 (0.511),138.3 (0.672),153.9 (0.659)

1440 (24.0),58.2 (0.566),70.3 (0.501),78.3 (0.466),86.1 (0.436),118.4 (0.499),130.7 (0.485)

2160 (36.0),32.3 (0.266),44.7 (0.266),52.9 (0.261),60.7 (0.254),100.0 (0.348),115.6 (0.355)

2880 (48.0),26.6 (0.195),39.1 (0.206),47.5 (0.207),55.4 (0.206),76.8 (0.237),92.7 (0.253)

4320 (72.0),10.1 (0.065),23.0 (0.105),31.5 (0.119),39.6 (0.127),62.7 (0.169),82.0 (0.196)

[PREBURST90_META]

Time Accessed,29 May 2019 01:22PM

Version,2018_v1

Note,Preburst interpolation methods for catchment wide preburst has been slightly altered. Point values remain unchanged.

[END_PREBURST90]

Interim Climate Change Factors

[CCF]

,RCP 4.5,RCP6,RCP 8.5

2030,0.869 (4.3%),0.783 (3.9%),0.983 (4.9%)

2040,1.057 (5.3%),1.014 (5.1%),1.349 (6.8%)

ARR2016.txt

2050,1.272 (6.4%),1.236 (6.2%),1.773 (9.0%)
2060,1.488 (7.5%),1.458 (7.4%),2.237 (11.5%)
2070,1.676 (8.5%),1.691 (8.6%),2.722 (14.2%)
2080,1.810 (9.2%),1.944 (9.9%),3.209 (16.9%)
2090,1.862 (9.5%),2.227 (11.5%),3.679 (19.7%)

[CCF_META]

Time Accessed,29 May 2019 01:22PM

Version,2019_v1

Note,ARR recommends the use of RCP4.5 and RCP 8.5 values. These have been updated to the values that can be found on the climate change in Australia website.

[END_CCF]

Probability Neutral Burst Initial Loss

[BURSTIL]

min (h)\AEP(%),50,20,10,5,2,1
60 (1.0),12.3,8.1,9.0,8.7,8.6,6.6
90 (1.5),11.8,8.1,9.5,9.7,9.5,7.0
120 (2.0),13.4,9.0,10.2,10.0,10.1,6.0
180 (3.0),13.8,9.3,10.6,10.1,8.9,4.3
360 (6.0),13.2,8.6,8.8,8.1,9.1,3.7
720 (12.0),17.6,12.2,12.2,10.6,11.9,3.1
1080 (18.0),18.3,13.5,14.6,12.0,13.3,3.8
1440 (24.0),21.5,15.5,15.8,13.8,14.6,4.4
2160 (36.0),24.2,18.4,18.4,15.9,16.6,6.9
2880 (48.0),27.2,22.0,21.1,22.9,19.4,9.5
4320 (72.0),29.4,25.5,25.5,25.7,21.8,10.5

[BURSTIL_META]

Time Accessed,29 May 2019 01:22PM

Version,2018_v1

Note,As this point is in NSW the advice provided on losses and pre-burst on the [NSW Specific Tab of the ARR Data Hub](#) is to be considered. In NSW losses are derived considering a hierarchy of approaches depending on the available loss information. Probability neutral burst initial loss values for NSW are to be used in place of the standard initial loss and pre-burst as per the losses hierarchy.

[END_BURSTIL]Transformational Pre-burst Rainfall

[PREBURST_TRANS]

min (h)\AEP(%),50,20,10,5,2,1
60 (1.0),28.7,32.9,32.0,32.3,32.4,34.4
90 (1.5),29.2,32.9,31.5,31.3,31.5,34.0
120 (2.0),27.6,32.0,30.8,31.0,30.9,35.0
180 (3.0),27.2,31.7,30.4,30.9,32.1,36.7
360 (6.0),27.8,32.4,32.2,32.9,31.9,37.3
720 (12.0),23.4,28.8,28.8,30.4,29.1,37.9
1080 (18.0),22.7,27.5,26.4,29.0,27.7,37.2
1440 (24.0),19.5,25.5,25.2,27.2,26.4,36.6
2160 (36.0),16.8,22.6,22.6,25.1,24.4,34.1
2880 (48.0),13.8,19.0,19.9,18.1,21.6,31.5
4320 (72.0),11.6,15.5,15.5,15.3,19.2,30.5

ARR2016.txt

[PREBURST_TRANS_META]

The transformational pre-burst is intended for software suppliers in the NSW area and is simply the Initial Loss - Burst Initial Loss. It is not appropriate to use these values if considering a calibrated initial loss.

[END_PREBURST_TRANS]

[ENDTXT]

Appendix 2

WBNM Results

DeeWhyBC_01.wbn

#####START_PREAMBLE_BLOCK#####|#####|#####|#####|#####
###|

Project Number:

Project Description:

#####END_PREAMBLE_BLOCK#####|#####|#####|#####|#####
###|

#####START_STATUS_BLOCK#####|#####|#####|#####|#####
###|

D:\Projects\2019 JN Projects\Dee Why BC [Flood]\WBNM

last edited on 13/06/2019 3:07:20 PM

by Andy Wiersma

2017_000

#####END_STATUS_BLOCK#####|#####|#####|#####|#####
###|

#####START_DISPLAY_BLOCK#####|#####|#####|#####|#####
###|

0.00 0.00 0.00 0.00

none

0.00 0.00 0.00 0.00 0.00

0.00

#####END_DISPLAY_BLOCK#####|#####|#####|#####|#####
###|

#####START_TOPOLOGY_BLOCK#####|#####|#####|#####|#####
###|

15

DWY04A 0.00 0.00 0.00 0.00 OUT

DeeWhyBC_01.wbn

DWY03A	0.00	0.00	0.00	0.00	DWY03B
DWY03B	0.00	0.00	0.00	0.00	DWY01C
DWY01A	0.00	0.00	0.00	0.00	DWY01B
DWY01B	0.00	0.00	0.00	0.00	DWY01C
DWY02A	0.00	0.00	0.00	0.00	DWY01C
DWY01C	0.00	0.00	0.00	0.00	DWY01D
DWY01D	0.00	0.00	0.00	0.00	OUT
STH02A	0.00	0.00	0.00	0.00	OUT
STH01A	0.00	0.00	0.00	0.00	STH01B
STH01B	0.00	0.00	0.00	0.00	OUT
COLL01A	0.00	0.00	0.00	0.00	COLL01B
COLL01B	0.00	0.00	0.00	0.00	COLL01C
COLL01C	0.00	0.00	0.00	0.00	OUT
OUT	0.00	0.00	0.00	0.00	SINK

#####END_TOPOLOGY_BLOCK#####|#####|#####|#####|#####
###|

#####START_SURFACES_BLOCK#####|#####|#####|#####|#####
###|

0.77
-99.90

DWY04A	9.169	50.0	1.60	0.10
DWY03A	0.389	90.0	1.60	0.10
DWY03B	4.448	55.0	1.60	0.10
DWY01A	1.707	55.0	1.60	0.10
DWY01B	1.436	15.0	1.60	0.10
DWY02A	1.092	95.0	1.60	0.10
DWY01C	1.536	15.0	1.60	0.10
DWY01D	3.027	25.0	1.60	0.10
STH02A	0.889	55.0	1.60	0.10
STH01A	13.396	60.0	1.60	0.10
STH01B	5.742	95.0	1.60	0.10
COLL01A	5.242	55.0	1.60	0.10
COLL01B	19.802	60.0	1.60	0.10
COLL01C	9.319	55.0	1.60	0.10
OUT	0.000	0.0	1.60	0.10

#####END_SURFACES_BLOCK#####|#####|#####|#####|#####

DeeWhyBC_01.wbn

|

#####START_FLOWPATHS_BLOCK#####|#####|#####|#####|#####
|

15

DWY04A

#####ROUTING

0.50

DWY03A

#####ROUTING

0.50

DWY03B

#####ROUTING

0.50

DWY01A

#####ROUTING

1.00

DWY01B

#####ROUTING

1.00

DWY02A

#####ROUTING

0.50

DWY01C

#####ROUTING

1.00

DWY01D

#####ROUTING

1.00

STH02A

#####ROUTING

0.50

STH01A

#####ROUTING

0.50

STH01B

#####ROUTING

DeeWhyBC_01.wbn

0.50
COLL01A
#####ROUTING
0.50
COLL01B
#####ROUTING
0.50
COLL01C
#####ROUTING
0.50
OUT
#####ROUTING
0.50
#####END_FLOWPATHS_BLOCK#####|#####|#####|#####
###|

#####START_LOCAL_STRUCTURES_BLOCK##|#####|#####|#####
###|
0
#####END_LOCAL_STRUCTURES_BLOCK###|#####|#####|#####
###|

#####START_OUTLET_STRUCTURES_BLOCK#|#####|#####|#####
###|
0
#####END_OUTLET_STRUCTURES_BLOCK###|#####|#####|#####
###|

#####START_STORM_BLOCK#####|#####|#####|#####|#####
###|
100
#####START_STORM#1
REC1 REC1
1.00
1.00

DeeWhyBC_01.wbn

#####START_RECORDED_RAIN

ARR16_Aep1_10min_4354

2 5.00

PERCENT

1

DEEWHY

0.00	0.00	36.10
60.56		
39.44		

#####END_RECORDED_RAIN

#####START_CALC_RAINGAUGE_WEIGHTS

#####END_CALC_RAINGAUGE_WEIGHTS

#####START_LOSS_RATES

DWY04A	0.00	0.72	0.00
DWY03A	0.00	0.72	0.00
DWY03B	0.00	0.72	0.00
DWY01A	0.00	0.72	0.00
DWY01B	0.00	0.72	0.00
DWY02A	0.00	0.72	0.00
DWY01C	0.00	0.72	0.00
DWY01D	0.00	0.72	0.00
STH02A	0.00	0.72	0.00
STH01A	0.00	0.72	0.00
STH01B	0.00	0.72	0.00
COLL01A	0.00	0.72	0.00
COLL01B	0.00	0.72	0.00
COLL01C	0.00	0.72	0.00
OUT	0.00	0.72	0.00

#####END_LOSS_RATES

#####START_RECORDED_HYDROGRAPHS

0

#####END_RECORDED_HYDROGRAPHS

#####START_IMPORTED_HYDROGRAPHS

0

#####END_IMPORTED_HYDROGRAPHS

#####END_STORM#1

#####START_STORM#2

Appendix 3

TUFLOW Results

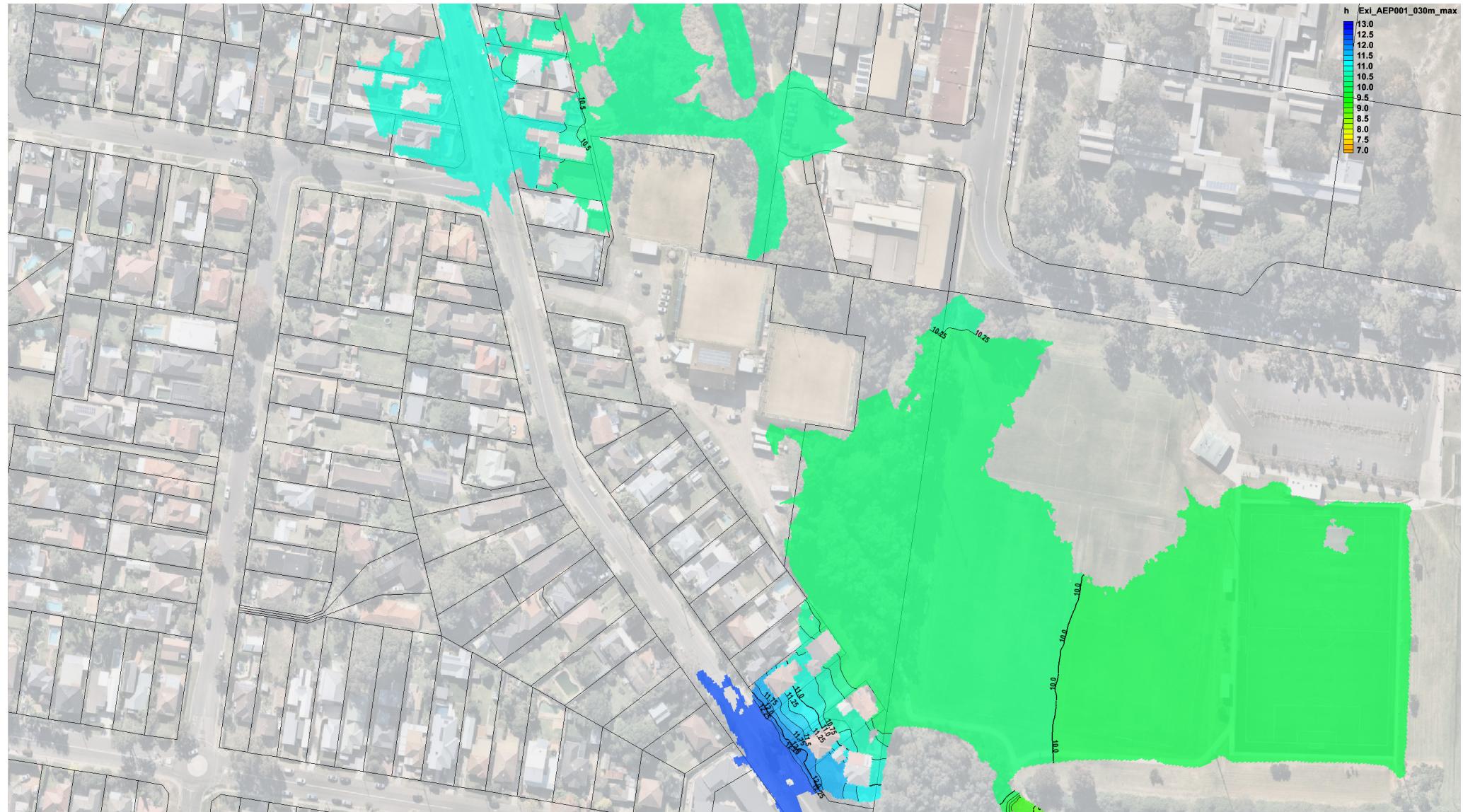


Figure A1: 1%AEP 30min Existing flood levels [mAHD] [all-clear](#)

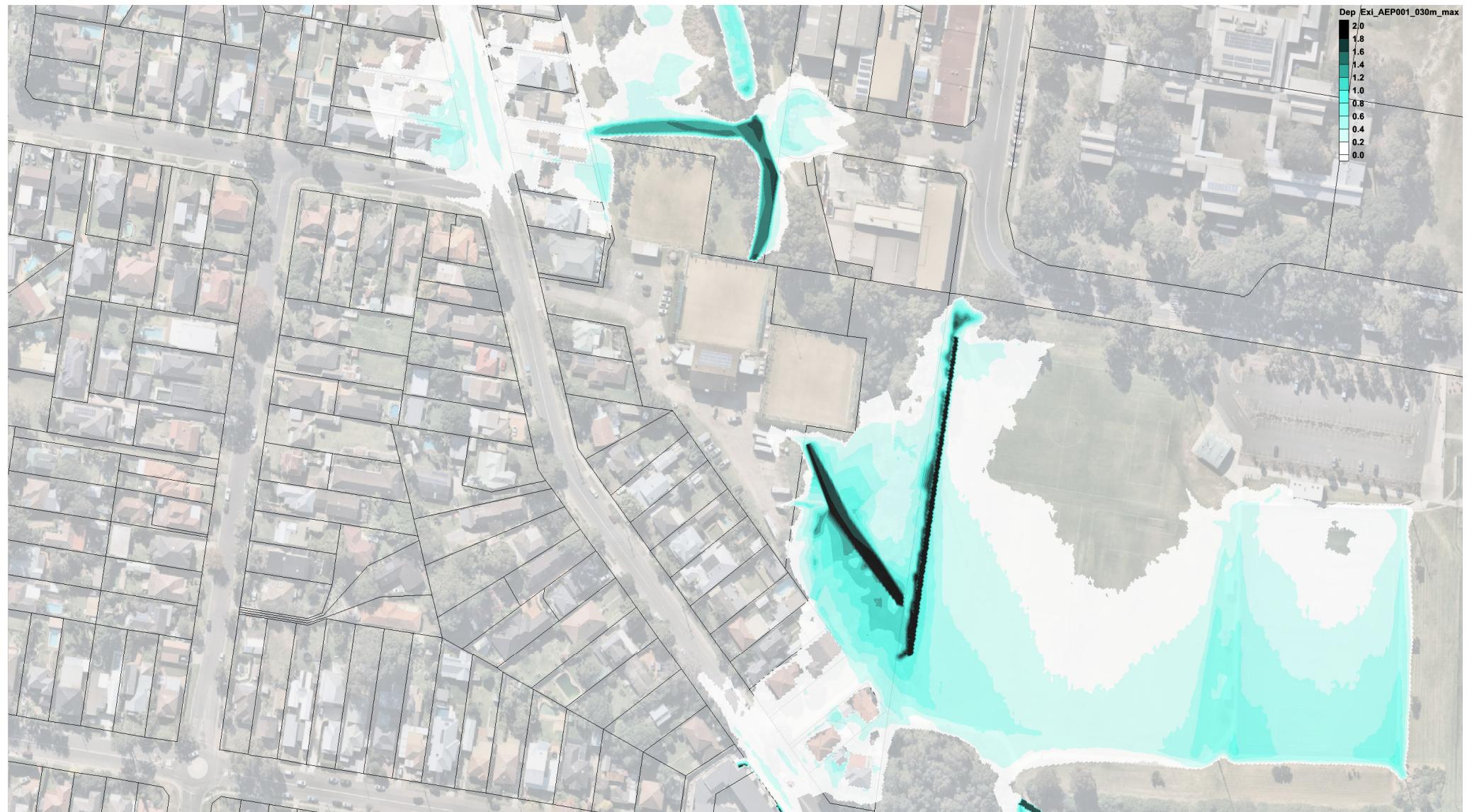


Figure A2: 1%AEP 30min Existing depths [m] [all-clear](#)

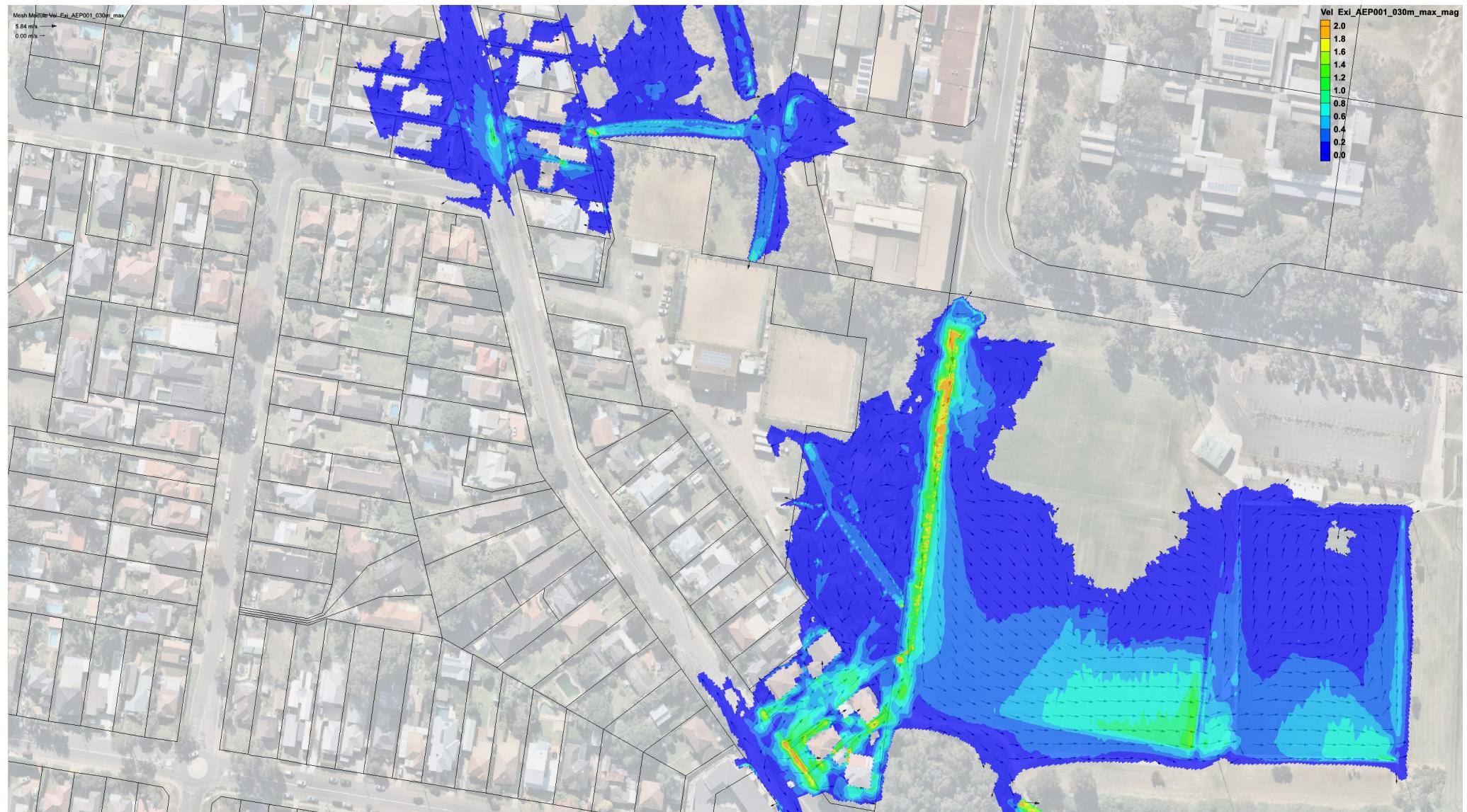


Figure A3: 1%AEP 30min Existing velocities [m/s] [all-clear](#)



Figure A4: 1%AEP 30min Existing flood Hazard [NSW FDM] [all-clear](#)

[light blue = low, dark blue =high]

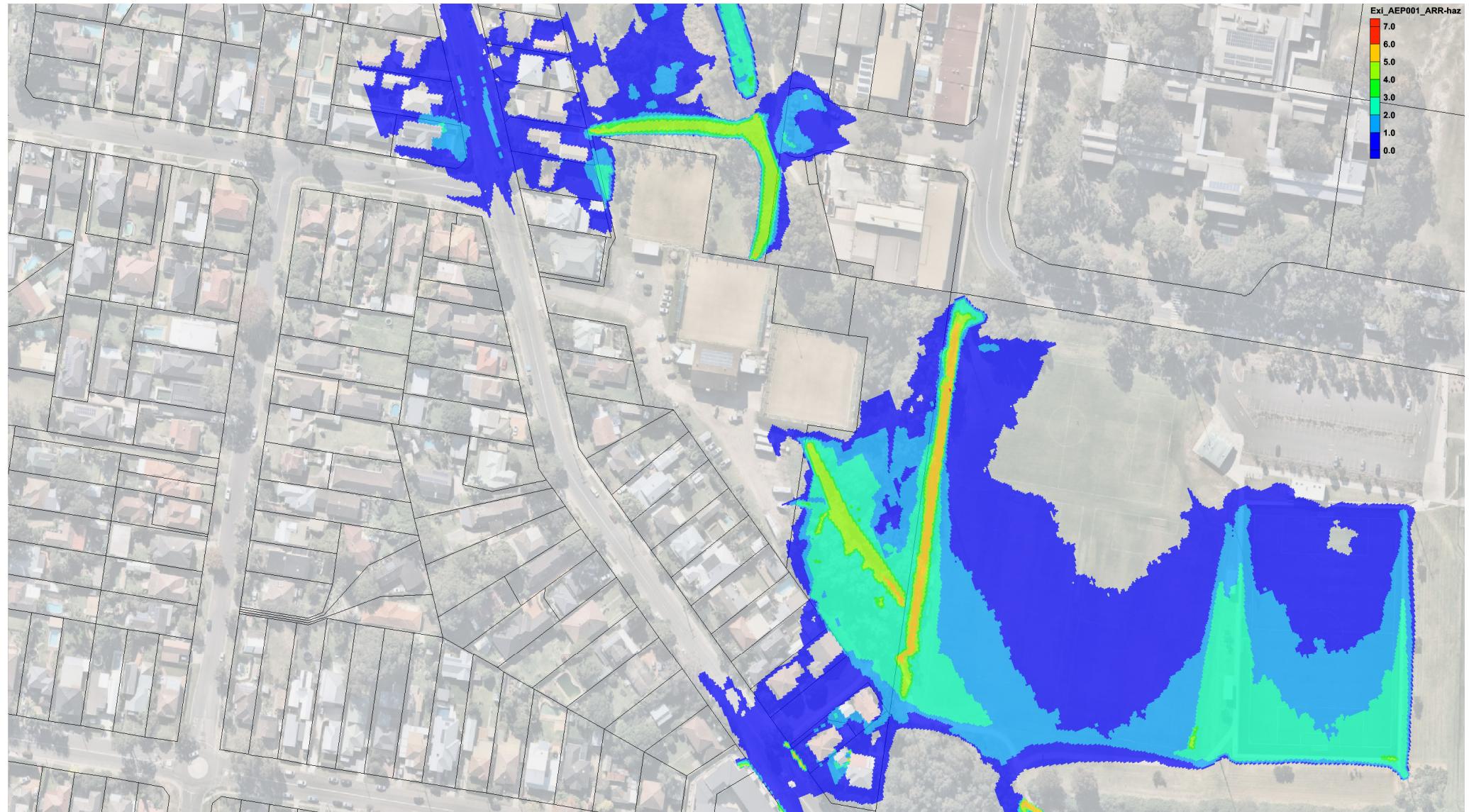


Figure A5: 1%AEP 30min Existing flood Hazard [ARR2019] [all-clear](#)

[H1=dark blue, H2=light blue, H3=aqua, H4 = green, H5=yellow, H6 = orange]

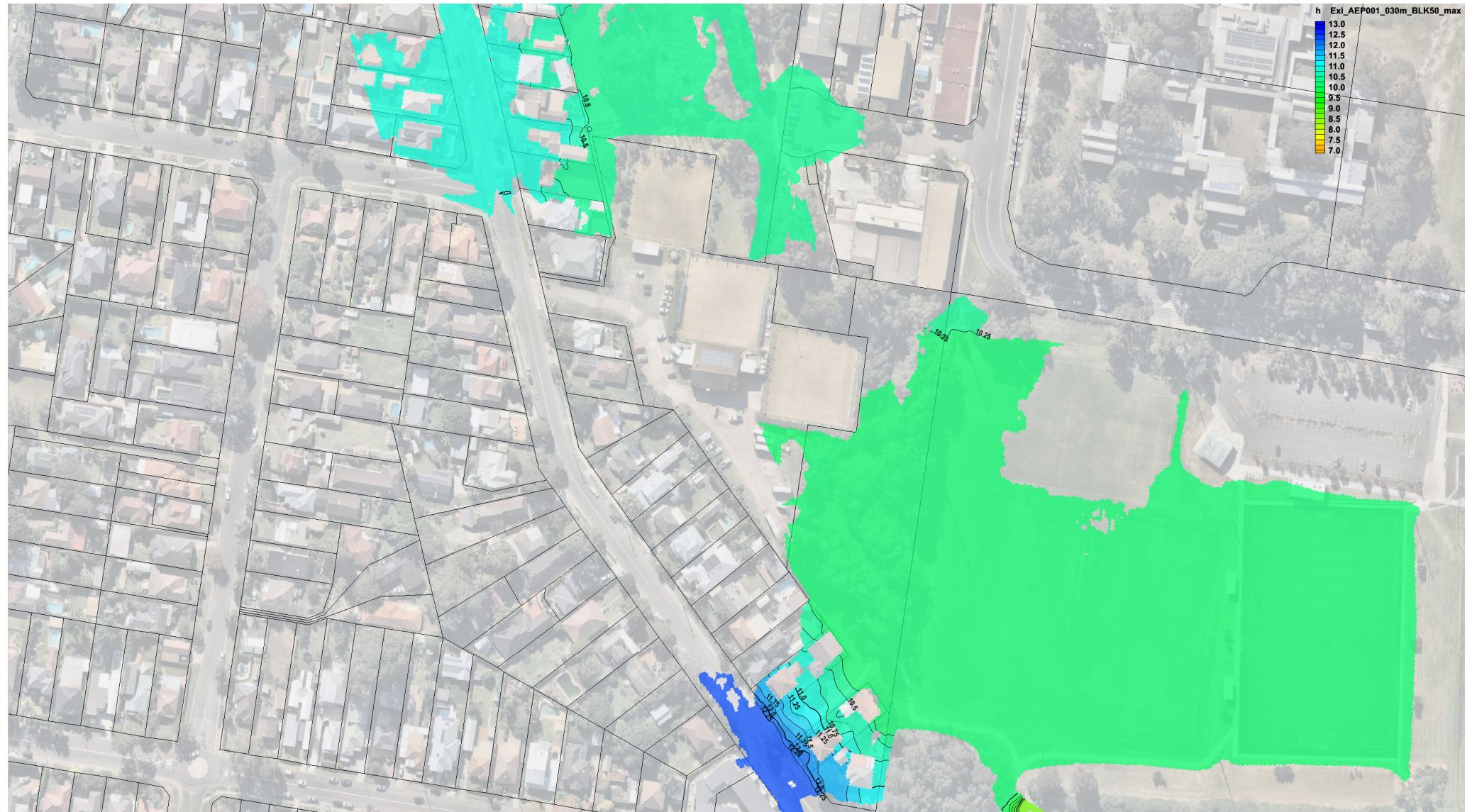


Figure A6: 1%AEP 30min Existing flood levels [mAHM] 50% blocked

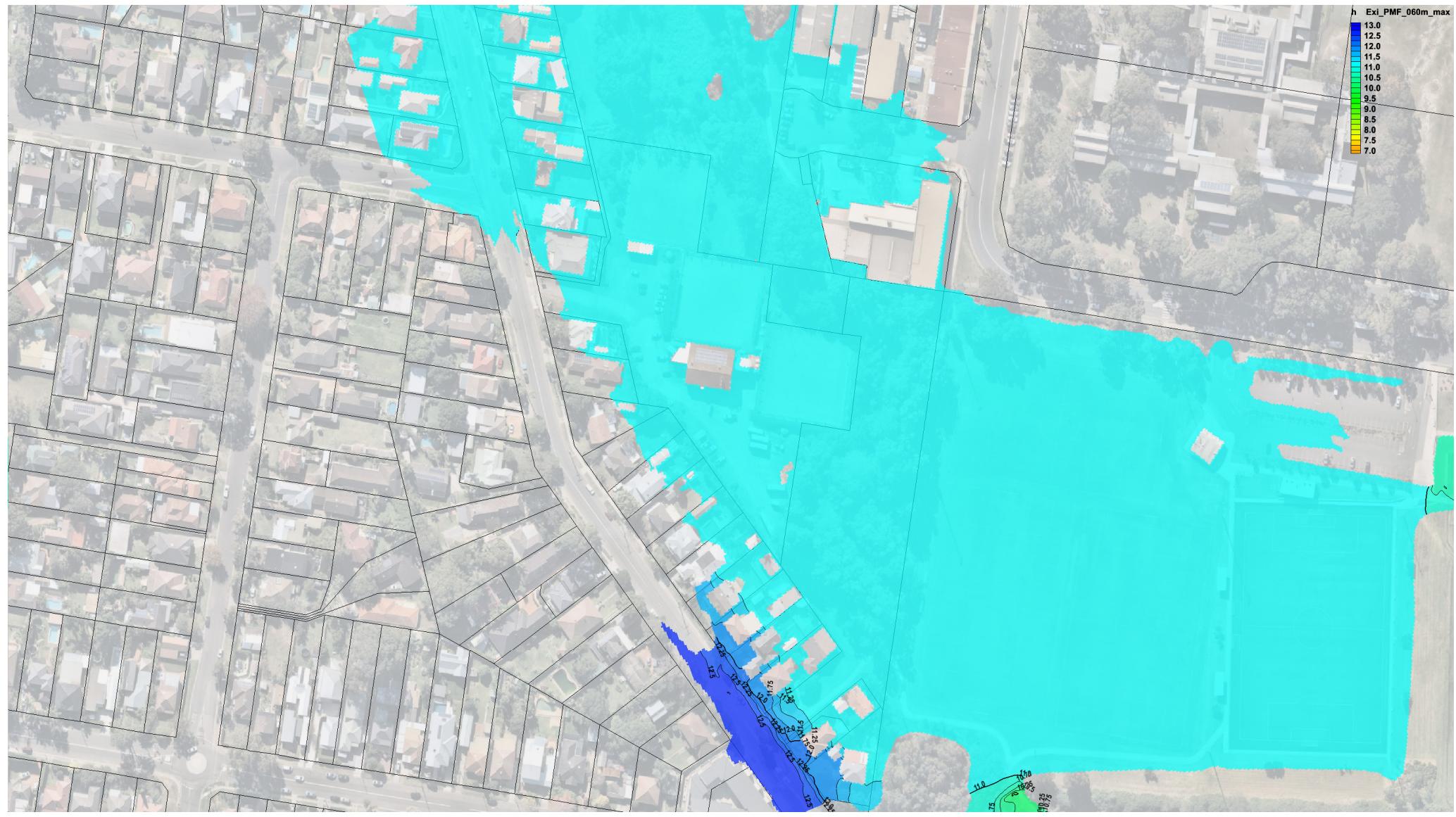


Figure A7: PMF 60min Existing flood levels [mAHM] *all-clear*

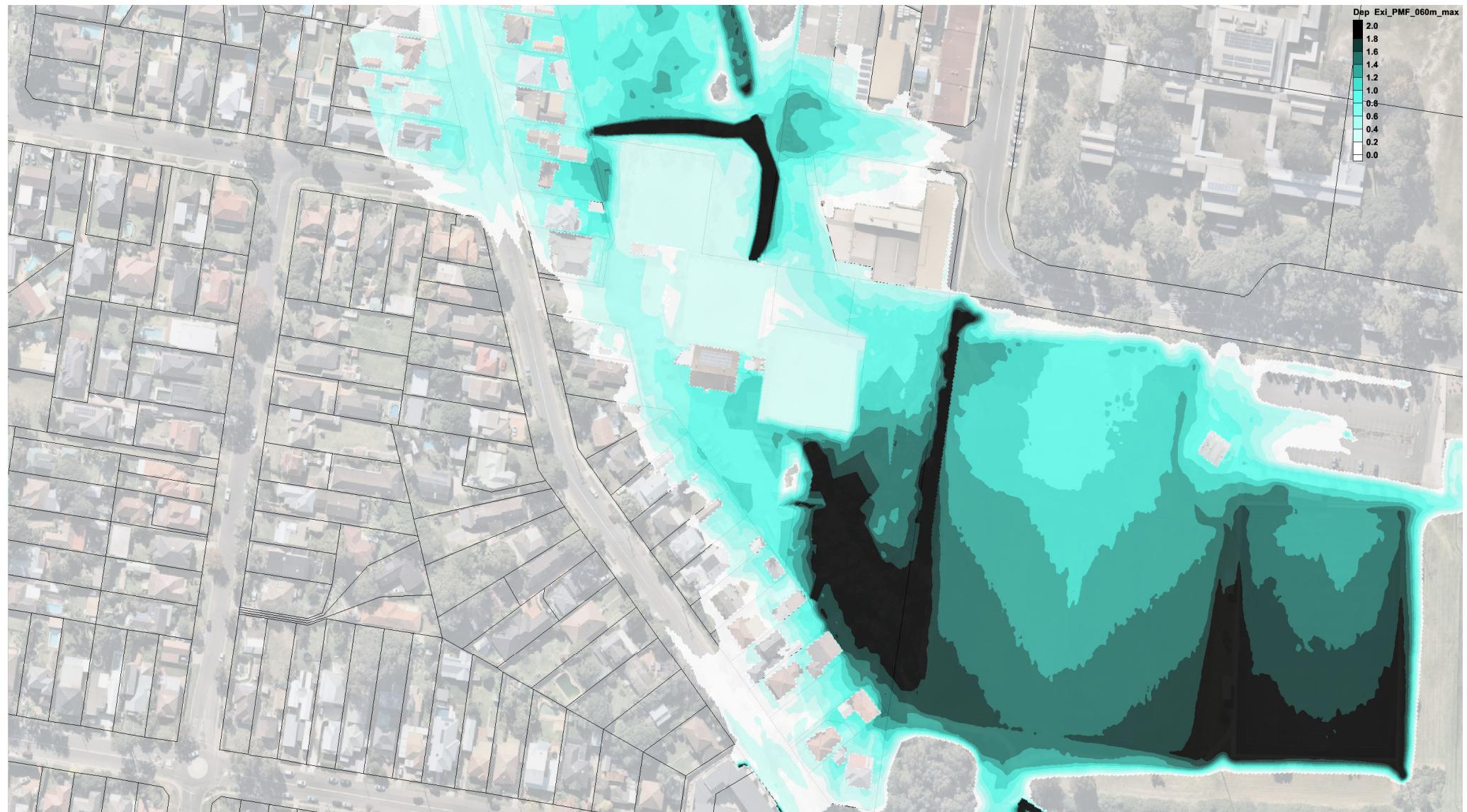


Figure A8: PMF 60min Existing depths [m] *all-clear*

Figures A9-A10 not used

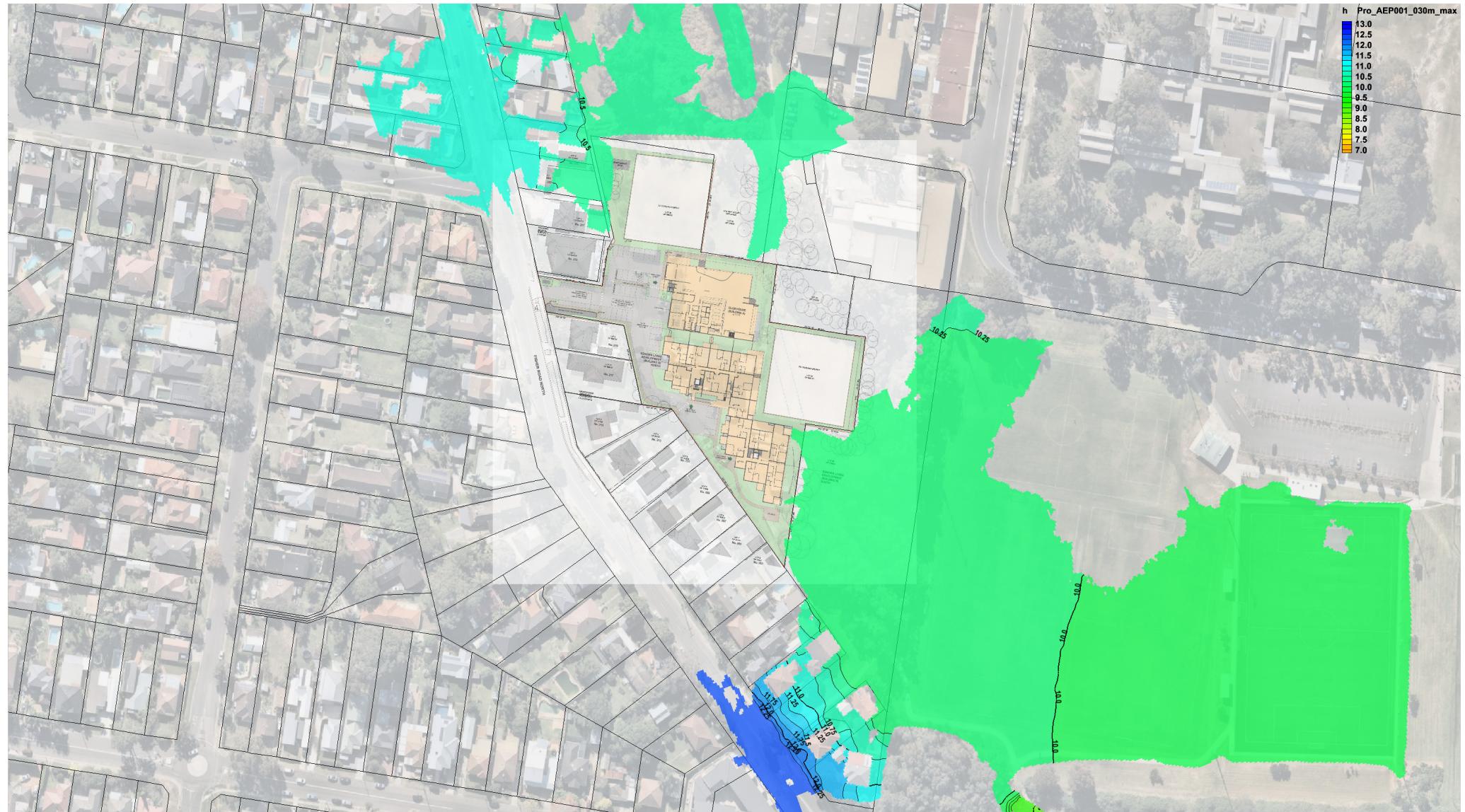


Figure A11: 1%AEP 30min Proposed flood levels [mAHM] [all-clear](#)



Figure A12: 1%AEP 30min Proposed depths [m] *all-clear*

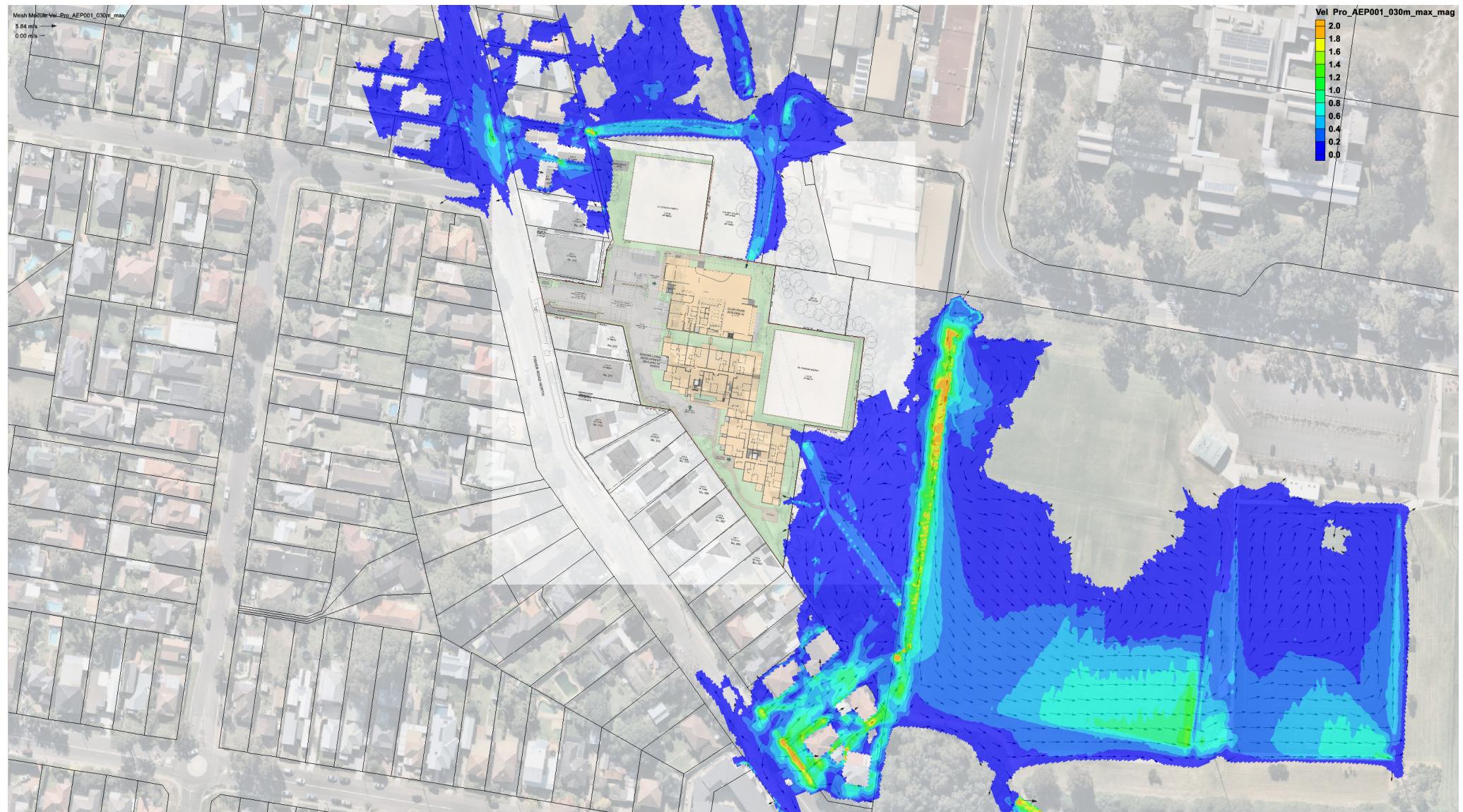


Figure A13: 1%AEP 30min Proposed velocities [m/s] [all-clear](#)

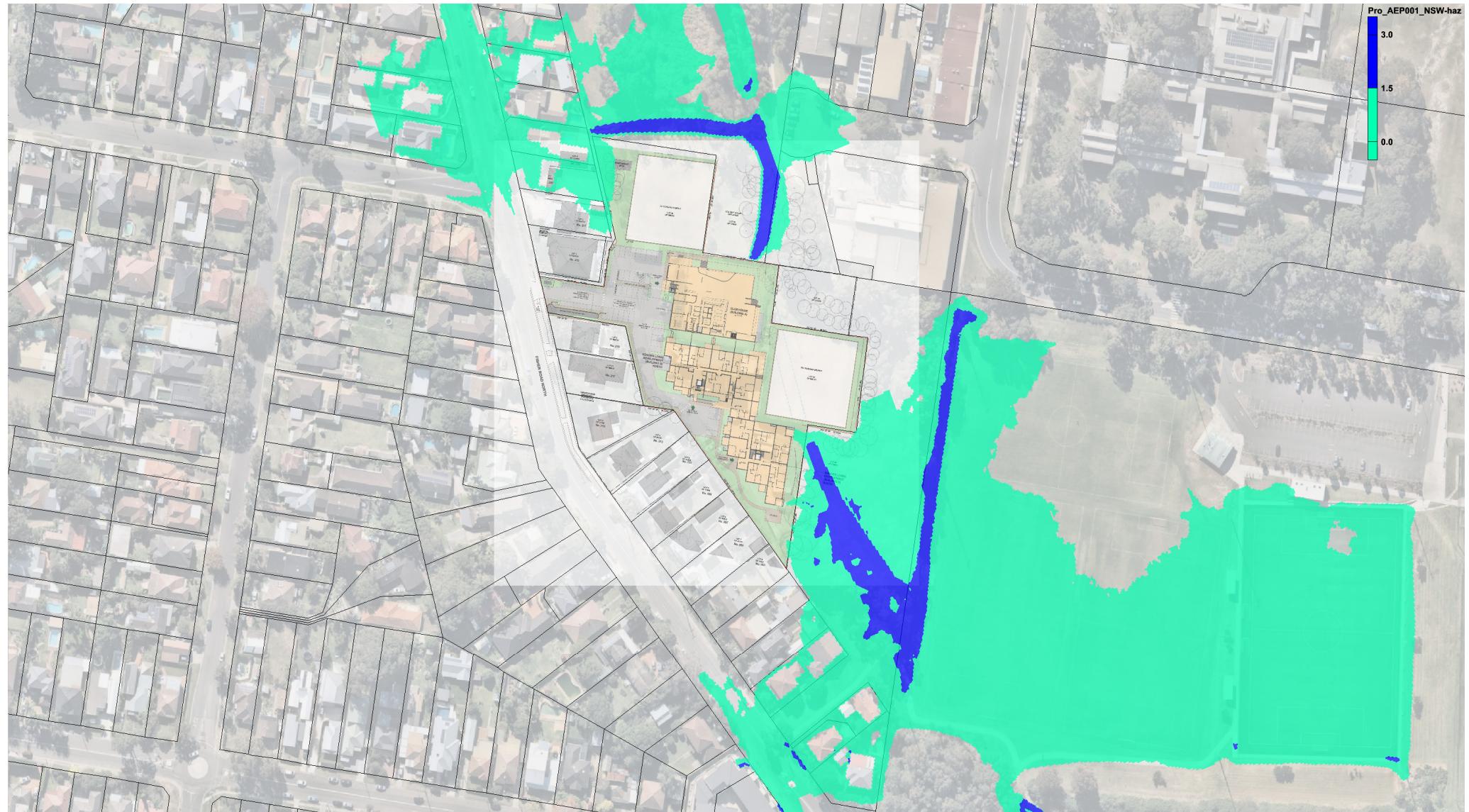


Figure A14: 1%AEP 30min Proposed flood Hazard [NSW FDM] [all-clear](#)

[light blue = low, dark blue = high]

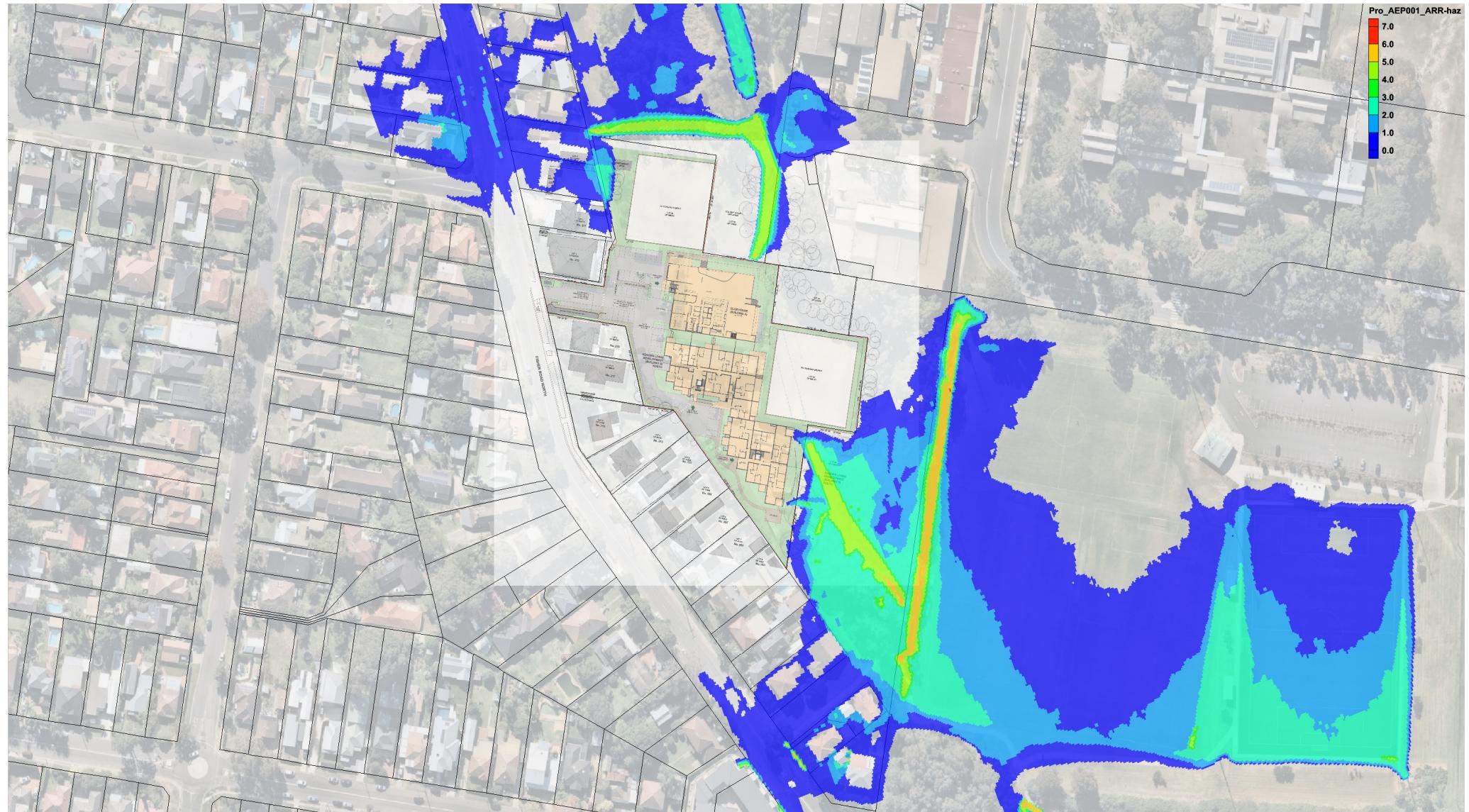


Figure A15: 1%AEP 30min Proposed flood Hazard [ARR2019] [all-clear](#)

[H1=dark blue, H2=light blue, H3=aqua, H4 = green, H5=yellow, H6 = orange]

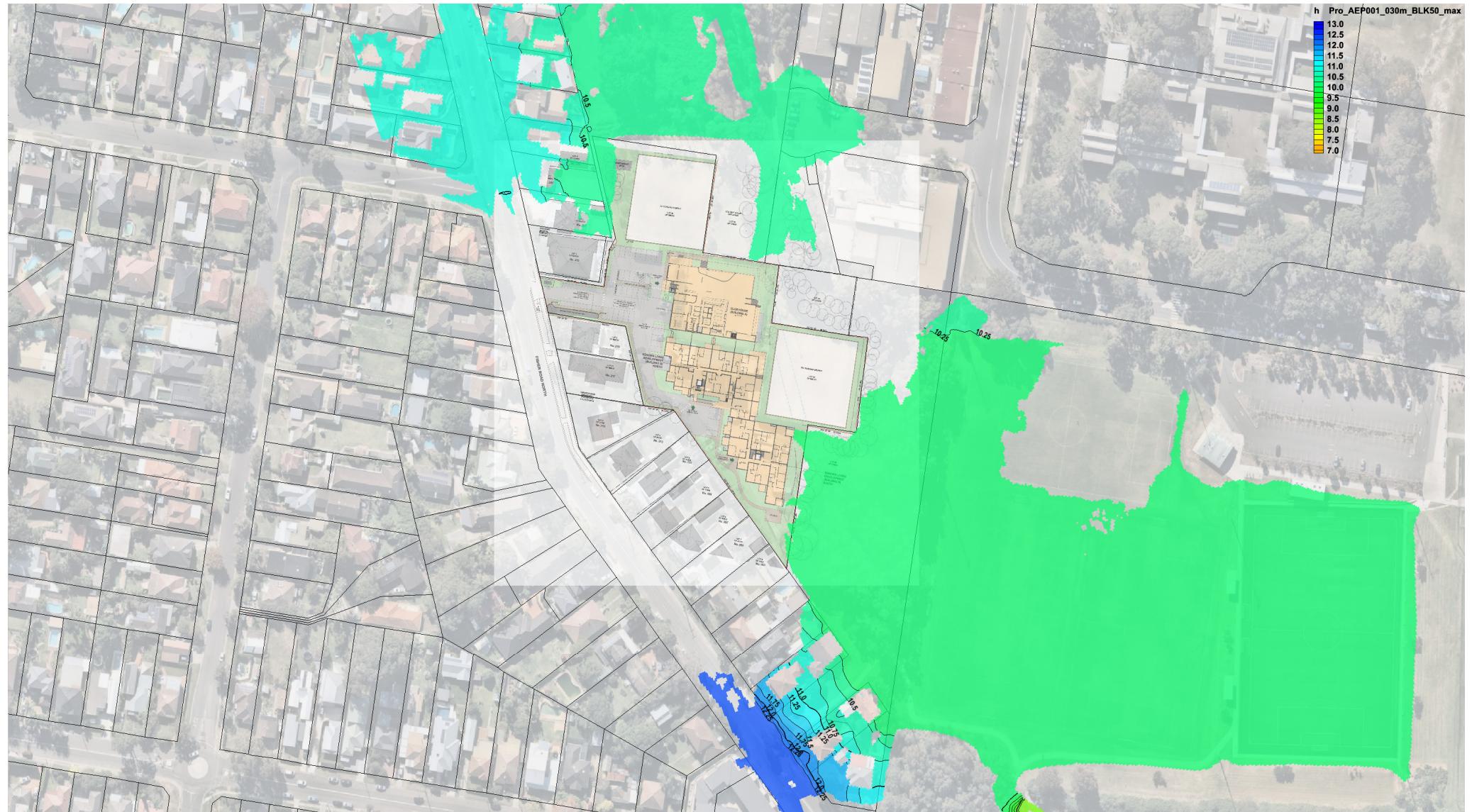


Figure A16: 1%AEP 30min Proposed flood levels [mAHD] **50% blocked**

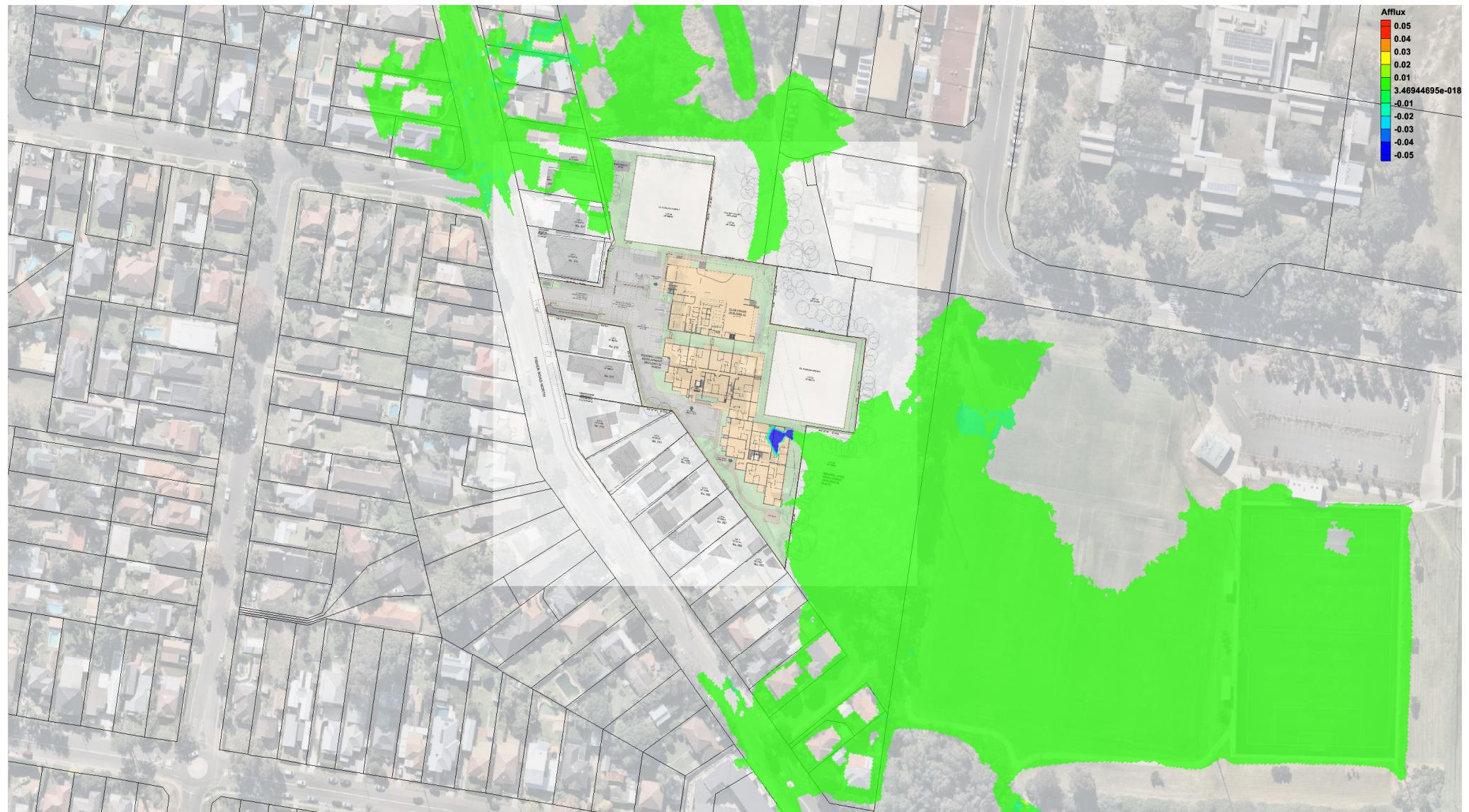


Figure A17: 1%AEP 30min Afflux [m] [all-clear](#)

[Proposed – Existing surfaces]

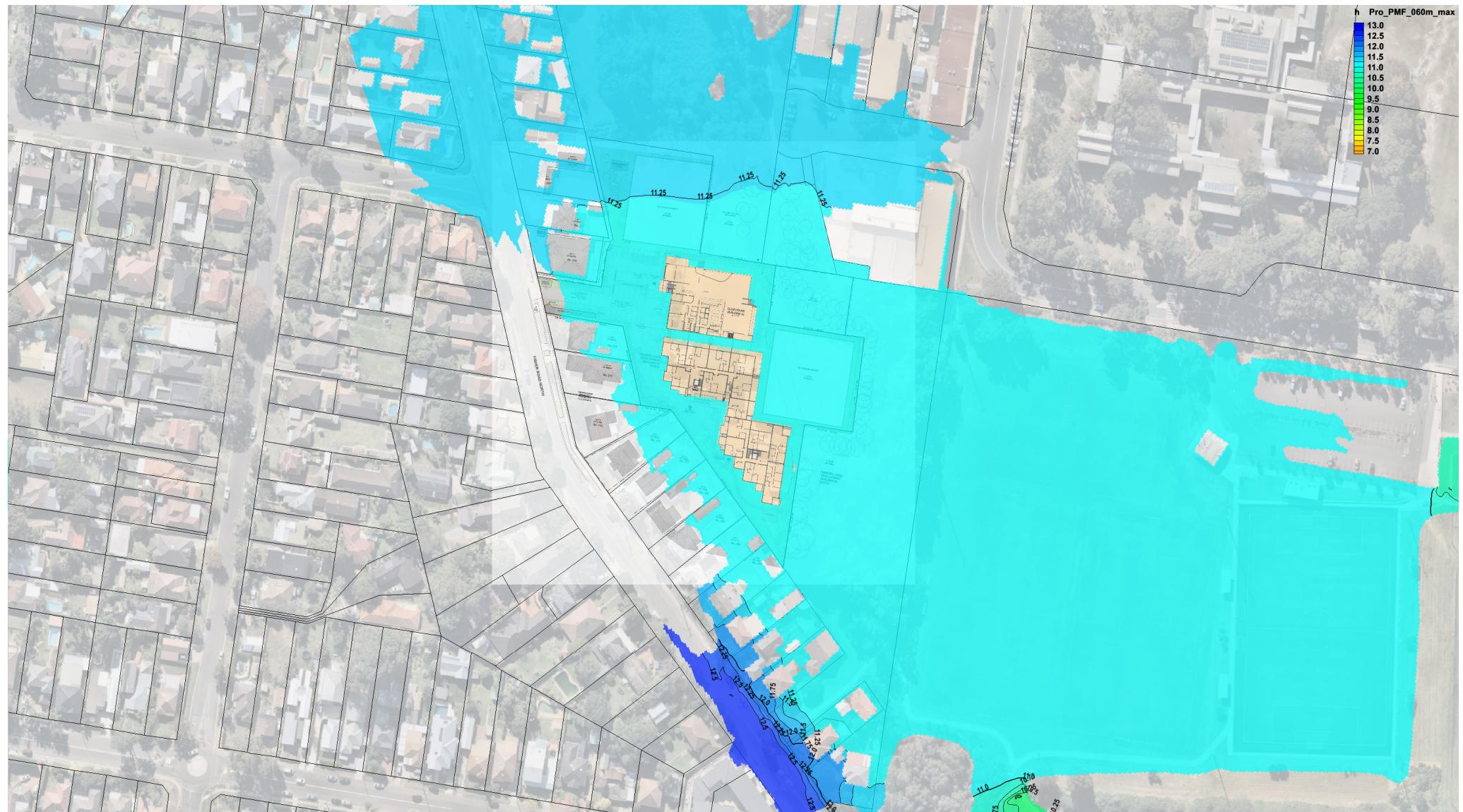


Figure A18: PMF 60min Proposed flood levels [mAHD] *all-clear*

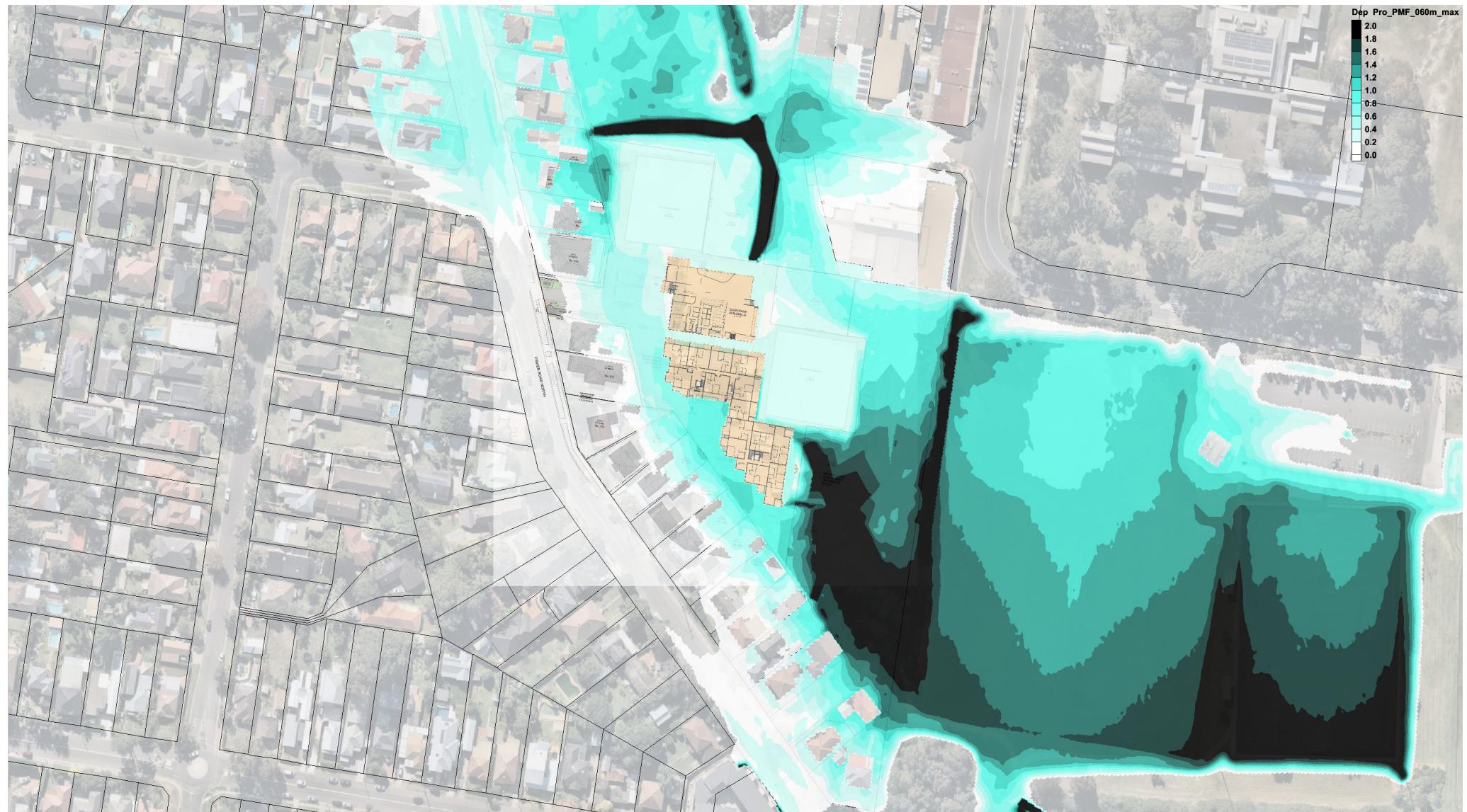


Figure A19: PMF 60min Proposed depths [m] *all-clear*